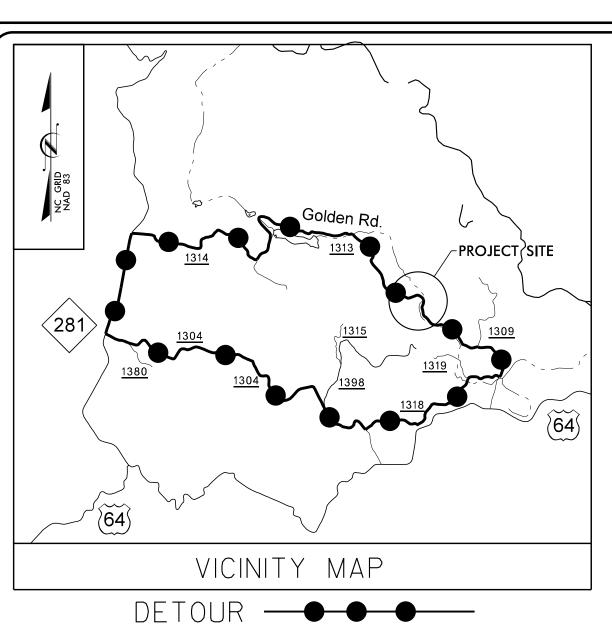
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14. 7B.

3

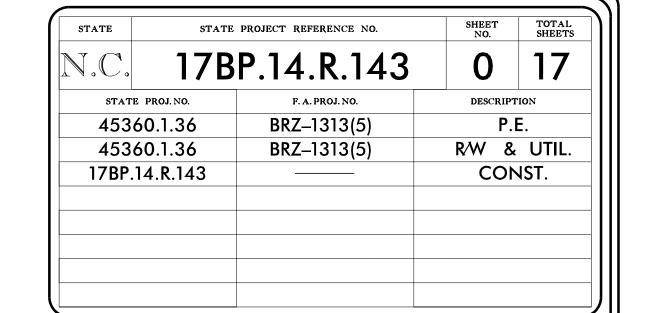


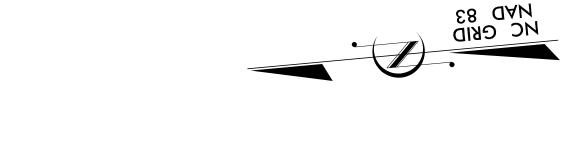
STATE OF NORTH CAROLINA

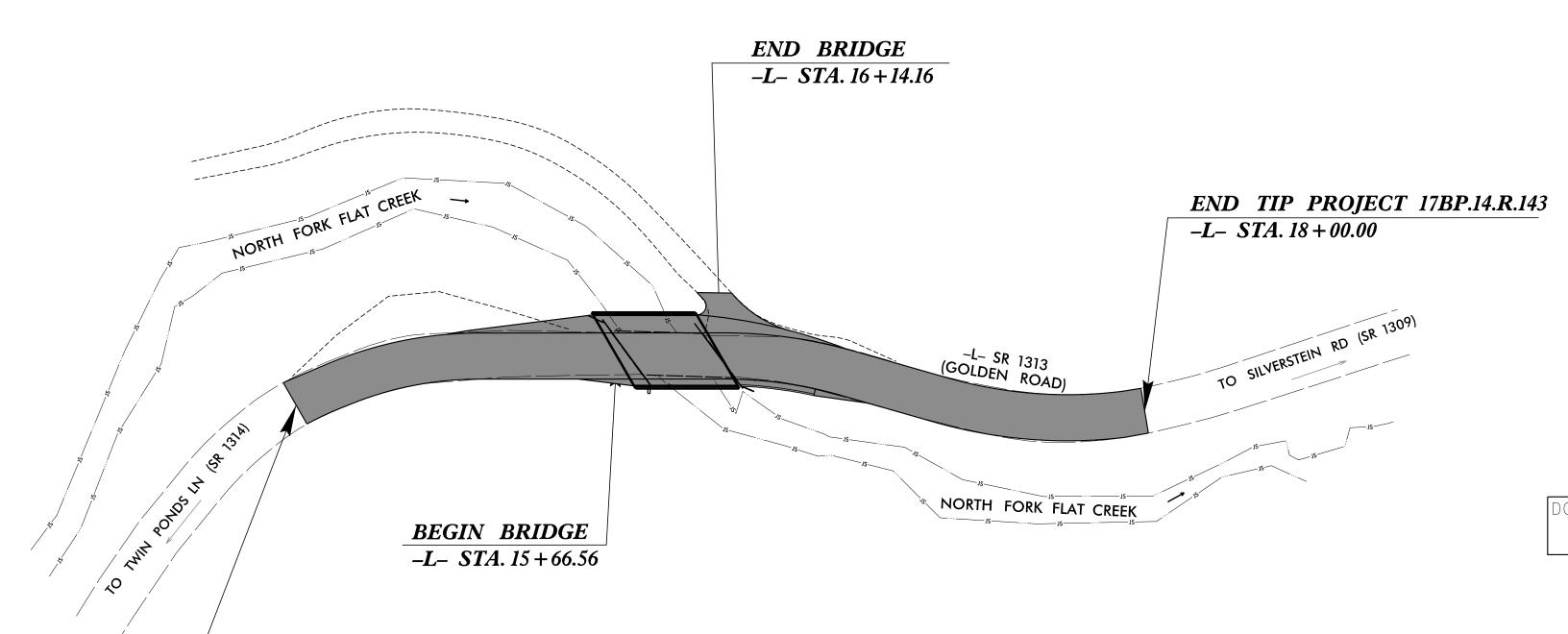
DIVISION OF HIGHWAYS

TRANSYLVANIA COUNTY

LOCATION: BRIDGE NO. 064 OVER NORTH FORK FLAT CREEK ON SR 1313 (GOLDEN ROAD)







DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

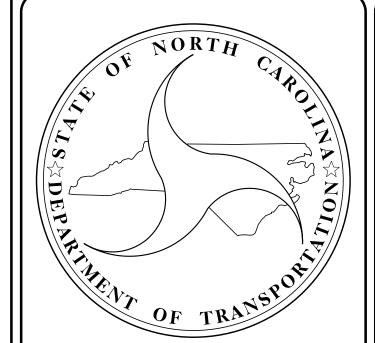


Asheville, ☐ Charleston, SC ■ North Carolina 828 - 253 - 2796

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☐ Boone, NC 828 · 355 · 9933 ☐ Tri-Cities, TN

☐ Spartanburg, S



DESIGN DATA

ADT 2010 = 290ADT 2025 = 580

> T = 6 %V = 20 MPH

FUNC CLASS = LOCAL SUB-REGIONAL TIER

PROJECT LENGTH

BEGIN TIP PROJECT 17BP.14.R.143

-L-STA. 14 + 25.00

LENGTH ROADWAY TIP PROJECT 17BP.14.R.143 = 0.062 MI. LENGTH STRUCTURE TIP PROJECT 17BP.14.R.143 = 0.009 MI. TOTAL LENGTH OF TIP PROJECT 17BP.14.R.143 = 0.071 MI.

Prepared in the Office of: VAUGHN & MELTON

1318-F PATTON AVE. ASHEVILLE NC, 28806 FOR THE NORTH CAROLINA DIVISION OF HIGHWAYS

2012 STANDARD SPECIFICATIONS

LETTING DATE:

HARDY WILLIS, PE PROJECT ENGINEER

CHRISTOPHER CORDELL, EI PROJECT DESIGN ENGINEER

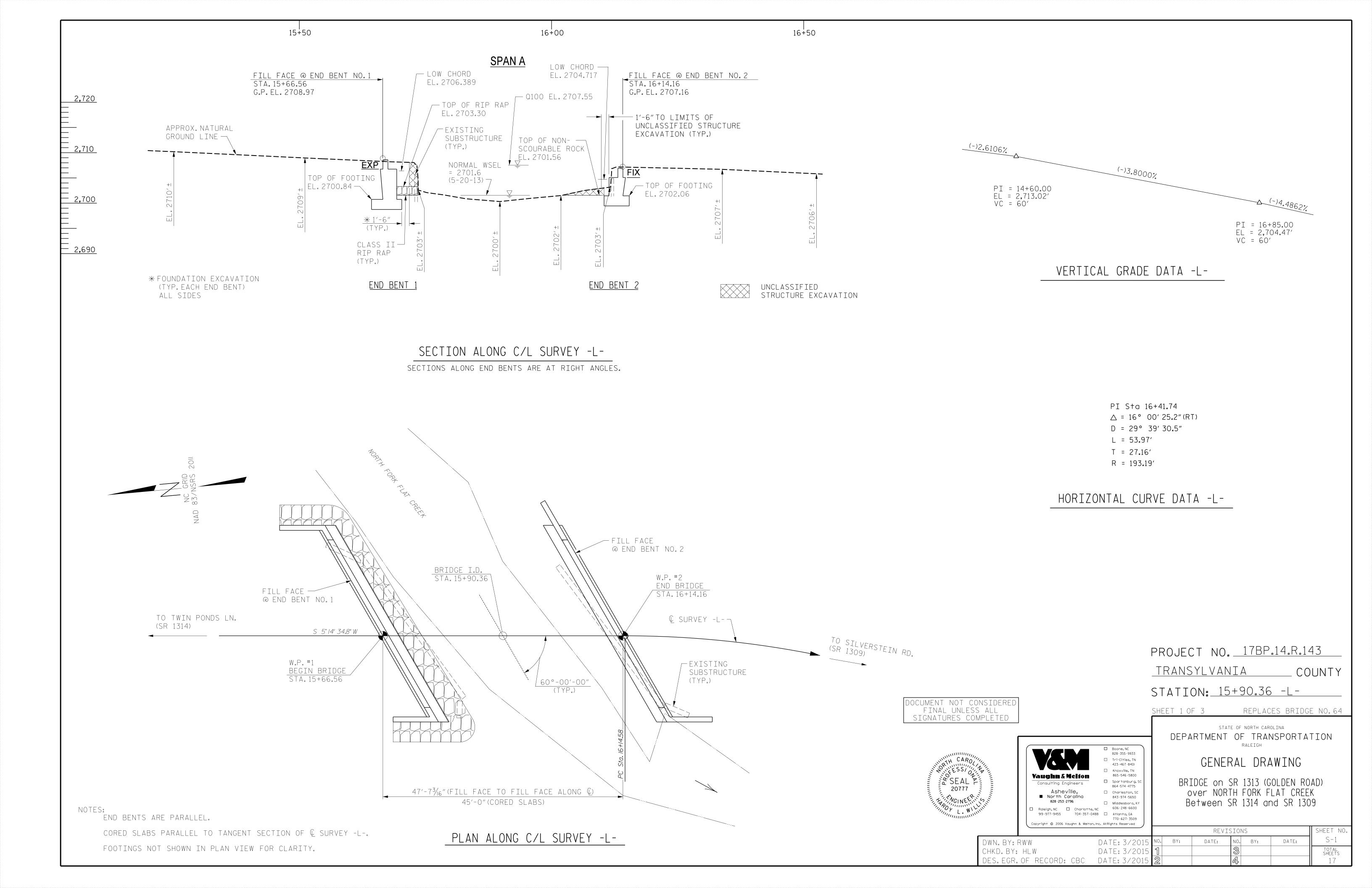
STRUCTURES MANAGEMENT UNIT 1000 BIRCH RIDGE DR.

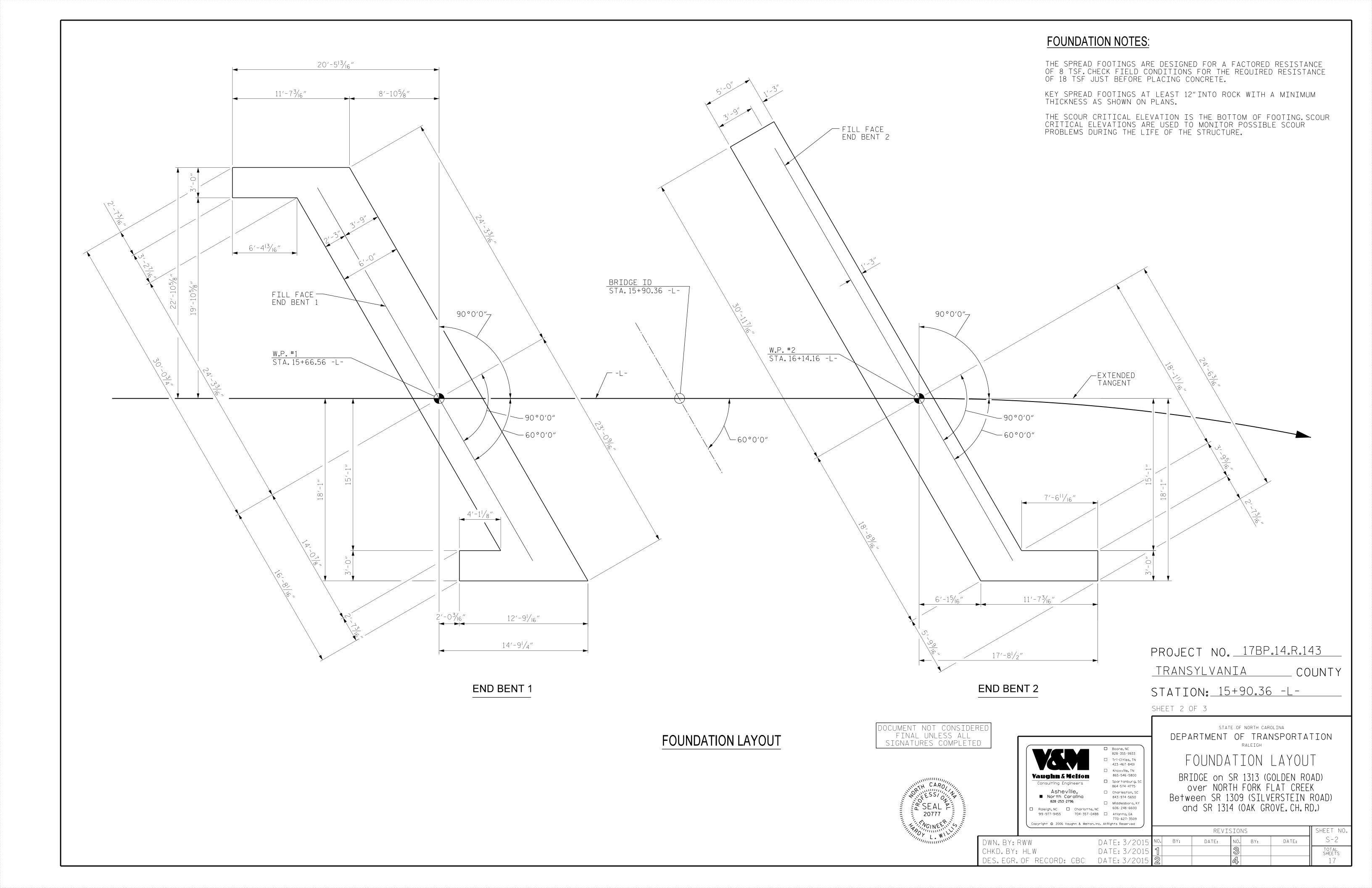
DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA **RALEIGH**, N.C. 27610

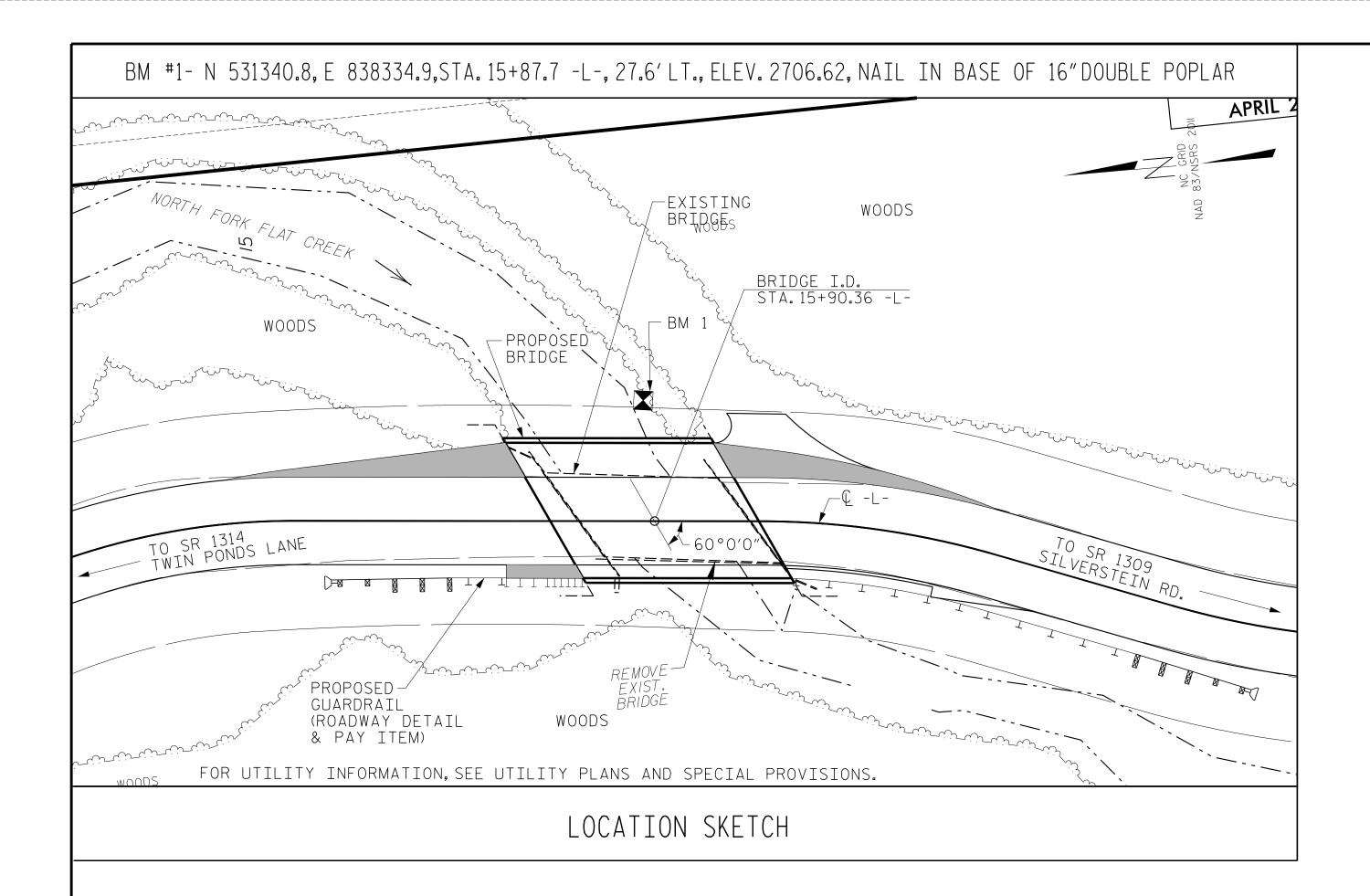
> JOSH DEYTON STATE DESIGN ENGINEER

DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION

DIVISION ADMINISTRATOR







GENERAL NOTES:

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC PERFORMANCE ZONE 1.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

THE EXISTING STRUCTURE, CONSISTING OF A SINGLE SPAN, 40.5 FOOT LONG TIMBER DECK ON STEEL I-BEAMS, AND A CLEAR ROADWAY WIDTH OF 19'-1", ON TIMBER CAPS AND PILES, AND LOCATED AT THE PROPOSED STRUCTURE SHALL BE REMOVED.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18-EVALUATING SCOUR AT BRIDGES".

- FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.
- FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.
- FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.
- FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
- FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 25 FT. EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 15+90.36 -L-."

HYDRAULIC DATA

DESIGN DISCHARGE = 650 CFS
DESIGN FREQUENCY = 25 YRS
DESIGN HW ELEVATION = 2706.0 FT
BASE DISCHARGE = 900 CFS
BASE FREQUENCY = 100 YRS
BASE HW ELEVATION = 2707.55 FT

OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE = 900 CFS OVERTOPPING FREQUENCY = 100 (±) YRS OVERTOPPING ELEVATION = 2707.6 FT DRAINAGE AREA = 1.92 SQ. MI.

			TO	TAL BILL	OF MATER:	IAL						
	REMOVAL OF EXISTING STRUCTURE	FOUNDATION EXCAVATION FOR END BENT	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE	REINFORCING STEEL		RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	PRE C	-0" x 1'-9" ESTRESSED CONCRETE DRED SLAB UNIT	ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES
	LUMP SUM	LUMP SUM	LUMP SUM	CU. YARDS	LBS.	LIN.FT.	TONS	SQ. YARDS	LUMP SUM	NO.	LIN.FT.	LUMP SUM
SUPERSTRUCTURE						90.29			LUMP SUM	11	495	LUMP SUM
END BENT 1		LUMP SUM	LUMP SUM	61.5	3,551		32	36				
END BENT 2		LUMP SUM	LUMP SUM	41.4	2,755							
TOTAL	LUMP SUM	LUMP SUM	LUMP SUM	102.9	6,306	90.29	32	36	LUMP SUM	11	495	LUMP SUM

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PROJECT NO. 17BP.14.R.143

TRANSYLVANIA COUNTY

STATION: 15+90.36 -L-

FOR
o,
over N
Between

SHEET 3 OF 3

GENERAL DRAWING
FOR BRIDGE on SR 1313
(GOLDEN ROAD) over NORTH FORK FLAT CREEK
Between SR 1314 and SR 1309

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

DWN.BY: RWW	DATE: 3/2015			REVI	SION	S		SHEET NO.
		NO.	BY:	DATE:	NO.	BY:	DATE:	S-3
CHKD. BY: HLW	DATE: 3/2015	1			3			TOTAL SHEETS
ENG. OF REC.: CBC	DATE: 3/2015	2			4			17

LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS STRENGTH I LIMIT STATE SERVICE III LIMIT STATE SHEAR MOMENT MOMENT DISTRIBUTION FACTORS (DF) ANCE END (ft) LIVELOAD FACTORS DISTRIBU Factors IVEL 1.205 1.75 0.2263 1.830 EL 21.923 0.6926 1.205 45′ EL 0.80 0.2263 HL-93(Inv) N/A 45′ 1.6 2.191 45′ EL 21.923 1.35 0.2263 1.562 2.372 45′ EL 21.923 0.6926 1.562 45′ 0.2263 HL-93(0pr) N/A EL 1.6 N/ADESIGN LOAD 51.222 1.75 0.2263 EL 1.423 36.000 1.423 2.249 45′ 21.923 0.6926 45′ EL 1.6 0.80 0.2263 2.687 45′ HS-20(Inv) 21.923 RATING 1.35 0.2263 0.2263 36.000 2.915 21.923 0.6926 1.844 66.399 45′ EL 1.844 45′ EL 1.6 HS-20(0pr) N/A 13.500 53.143 0.2263 SNSH 3.937 1.4 0.2263 5.444 45′ EL 21.923 0.6926 3.937 45′ EL 1.6 0.80 5.244 45′ 21.923 57.701 1.4 0.2263 20.000 2.885 45′ EL 21.923 0.6926 2.885 45′ EL 1.6 0.80 0.2263 4.236 45′ SNGARBS2 4.434 21.923 17.538 0.6926 22.000 2.717 59.772 1.4 0.2263 45′ EL 2.717 45′ EL 1.6 0.80 0.2263 4.173 45′ 21.923 SNAGRIS2 4.344 27.250 1.973 53.754 21.923 0.6926 1.4 0.2263 2.739 45′ EL 1.973 45′ EL 1.6 0.80 0.2263 2.618 21.923 SNCOTTS3 45′ 21.923 | 0.6926 0.2263 SNAGGRS4 34.925 1.703 59.488 0.2263 EL 1.703 45′ 0.80 2.313 45′ 21.923 2.416 45′ EL 1.6 1.4 0.2263 1.761 EL SNS5A 35.550 1.761 62.599 2.350 45′ 21.923 0.6926 45′ EL 1.6 0.80 0.2263 2.252 45′ 21.923 39.950 65.893 1.4 0.2263 EL 21.923 0.6926 1.649 45′ EL 1.6 0.80 0.2263 2.136 45′ SNS6A 1.649 2.228 45′ 21.923 42.000 1.643 69.017 1.4 0.2263 2.108 45′ EL 21.923 0.6926 1.643 45′ EL 1.6 0.80 0.2263 2.024 45′ 21.923 SNS7B EL LEGAL LOAD 0.2263 2.605 TNAGRIT3 33.000 1.921 63.403 0.2263 2.722 EL 21.923 0.6926 1.921 45′ 0.80 21.923 45′ EL 1.6 RATING 1.4 0.2263 TNT4A 33.075 1.841 60.904 2.739 45′ EL 21.923 0.6926 1.841 45′ EL 1.6 0.80 0.2263 2.633 21.923 TNT6A 41.600 1.789 74.436 1.4 0.2263 2.312 EL 21.923 0.6926 1.789 45′ EL 1.6 0.80 0.2263 2.211 45′ 21.923 1.4 0.2263 EL 21.923 0.6926 1.656 45′ EL 1.6 0.80 0.2263 2.254 TNT7A 42.000 1.656 69.533 2.350 45′ 45′ 21.923 EL 21.923 0.6926 0.2263 42.000 1.376 66.192 0.2263 45′ EL 1.576 45′ 0.80 2.346 21.923 TNT7B 2.443 EL 1.6 45′ 65.215 1.4 0.2263 0.2263 1.517 21.923 0.6926 1.517 TNAGRIT4 43.000 2.337 45′ EL 45′ EL 1.6 0.80 2.234 45′ 21.923 1.551 45.000 1.4 0.2263 EL 21.923 0.6926 45′ EL 1.6 0.80 0.2263 2.080 TNAGT5A 69.802 2.172 45′ 45′ 21.923 3 | 1.438 | 64.703 | 1.4 | 0.2263 | 2.118 | 45' | EL | 21.923 | 0.6926 | **1.438** | 1.6 | 0.80 | 0.2263 | 2.030 | 45' | EL | 21.923 |

FOR SPAN A

LOAD FACTORS:

DESIGN	LIMIT STATE	$\gamma_{ extsf{DC}}$	$\gamma_{\sf DW}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

2

3.

4

(#) CONTROLLING LOAD RATING

(1) DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

 $\langle 3 \rangle$ Legal load rating **

** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

PROJECT NO. 17BP.14.R.143

TRANSYLVANIA COUNTY

STATION: 15+90.36 -L-

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALETCH

LRFR SUMMARY FOR 45' CORED SLAB UNIT 60° SKEW SKEW

(NON-INTERSTATE TRAFFIC)

	SHEET NO.				
BY:	DATE:	NO.	BY:	DATE:	S-4
		33			TOTAL SHEETS
		ΔL			17

RFR SUMMARY

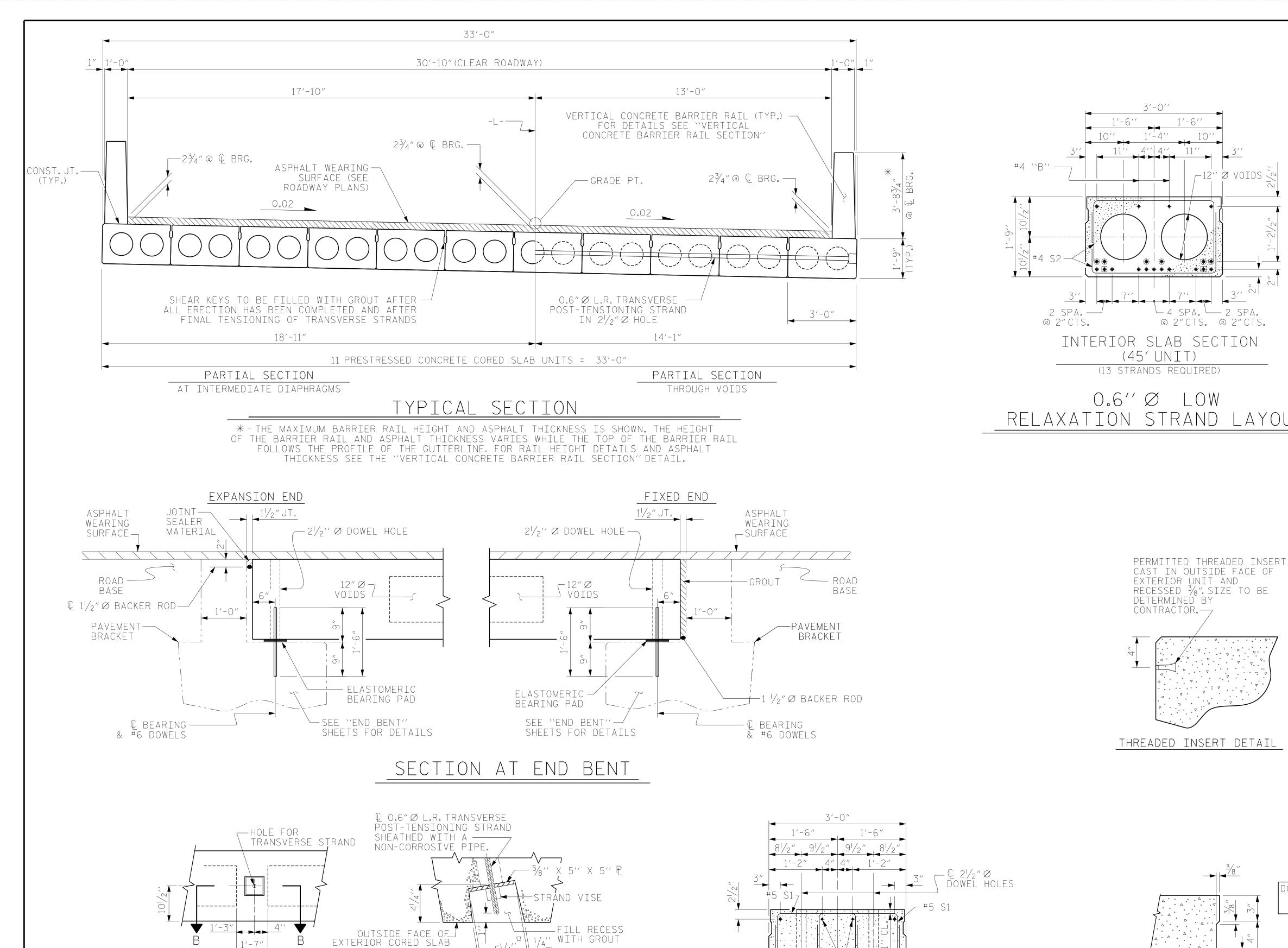
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OCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

ASSEMBLED BY: RWW DATE: 3/2015 CHECKED BY: HLW DATE: 3/2015 DRAWN BY: CVC 6/10

CHECKED BY : DNS 6/10

STD. NO. 21LRFR1_60&120S_45L



#5 S1 —

#5 S1 —

END ELEVATION

UNIT SIMILAR EXCEPT SHEAR KEY LOCATION.

ELEVATION VIEW

DATE: 3/2015

DATE: 3/2015

DES.EGR.OF RECORD: $\overline{\sf CBC}$

DRAWN BY :

CHECKED BY :

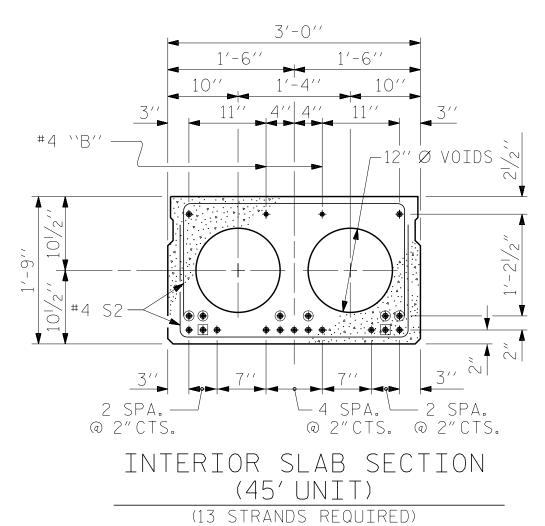
RWW

HLW

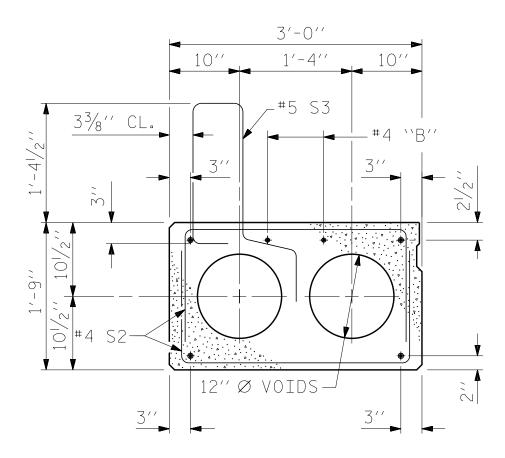
SECTION B-B

GROUTED RECESS AT END OF

POST-TENSIONED STRAND OF CORED SLABS



RELAXATION STRAND LAYOUT



EXT. SLAB SECTION (FOR PRESTRESSED STRAND LAYOUT, SEE INTERIOR SLAB SECTION.)

- BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 6'-O"FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.
- BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 2'-O"FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.
- OPTIONAL FULL LENGTH DEBONDED STRANDS. THESE STRANDS ARE NOT REQUIRED. IF THE FABRICATOR CHOOSES TO INCLUDE THESE STRANDS IN THE CORED SLAB UNIT, THE STRANDS SHALL BE DEBONDED FOR THE FULL LENGTH OF THE UNIT AT NO ADDITIONAL COST. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.

DEBONDING LEGEND

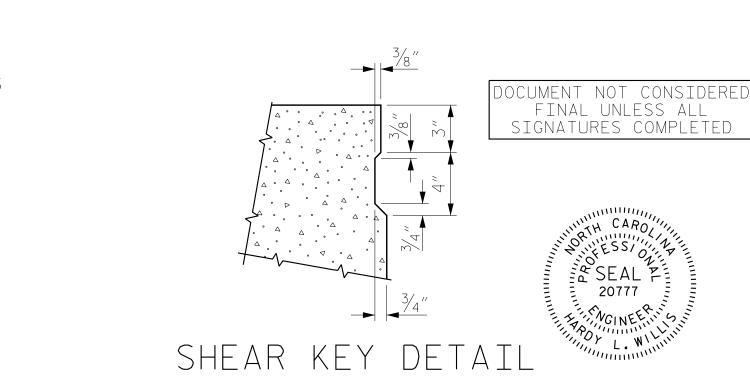
PROJECT NO. <u>178P</u>.14.R.143 TRANSYLVANIA STATION: 15+90.36 -L-

SHEET 1 OF 3 STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION STANDARD

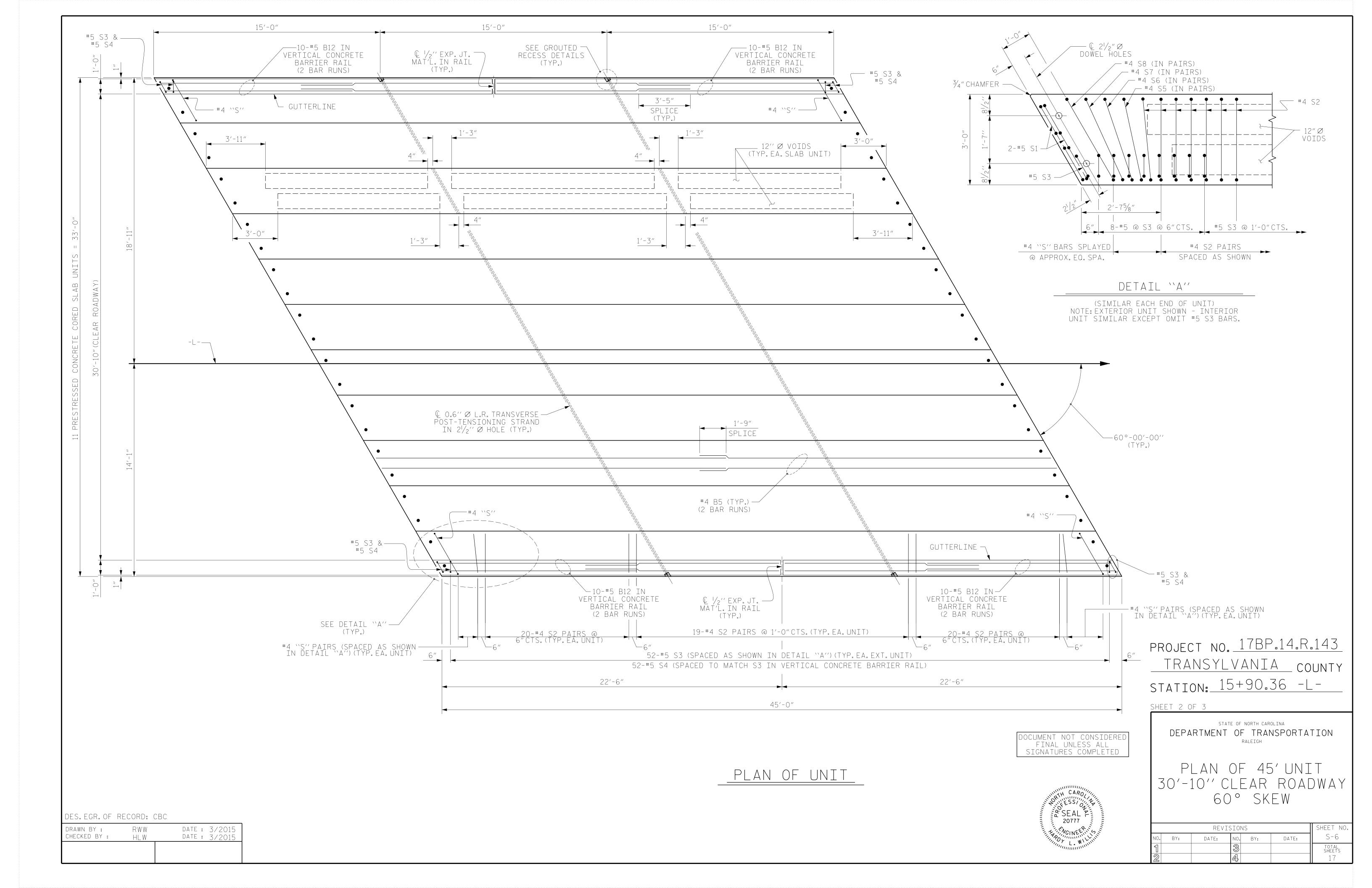
3'-0'' X 1'-9'' PRESTRESSED CONCRETE CORED SLAB UNIT 60° SKEW

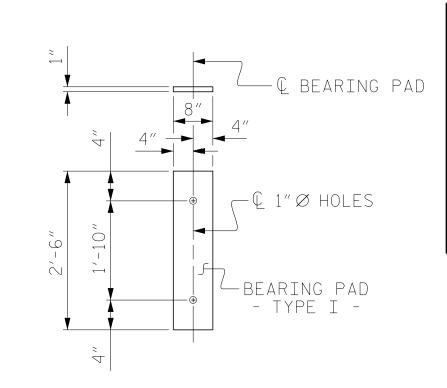
SHEET NO REVISIONS S-5 DATE: DATE: BY: 10. BY: TOTAL SHEETS



20777

NOTE: OMIT SHEAR KEY ON OUTSIDE FACE OF EXTERIOR CORED SLABS. SHOWING PLACEMENT OF DOUBLE STIRRUPS AND LOCATION OF DOWEL HOLES. (STRAND LAYOUT NOT SHOWN.)
INTERIOR SLAB UNIT SHOWN-EXTERIOR SLAB





BI	LL OF MATERIAL FOR VERTI	CAL CONC	RETE	BARR	RIER R	AIL
BAR	BARS PER PAIR OF EXTERIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT
	45' UNIT					
 ₩ B12	80	80	#5	STR	13'-0"	1085
* S4	108	108	#5	2	7'-2"	807
* EPOX	Y COATED REINFORCING STEEL			LBS.		1892
CLASS	AA CONCRETE			CU.YDS.	1	11.5
TOTAL	VERTICAL CONCRETE BARRIER RAIL			LN.FT.		90.29

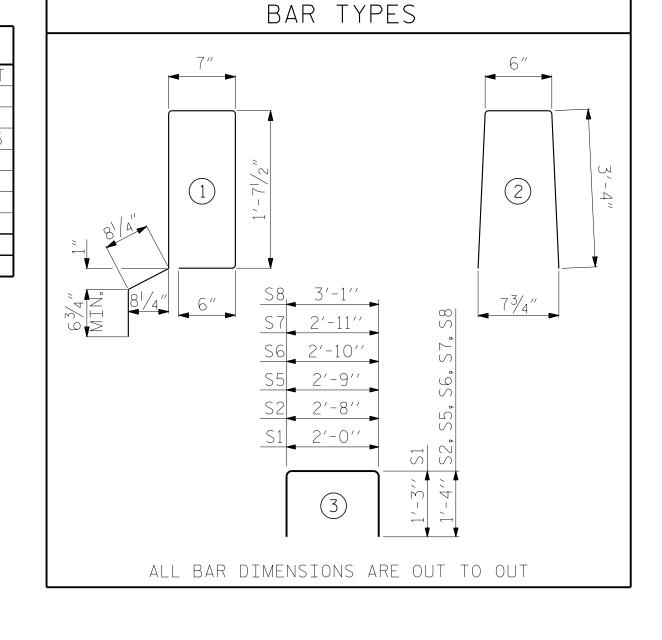
FIXED END (TYPE I - 22 REQ'D)

ELASTOMERIC BEARING DETAILS

ELASTOMER IN ALL BEARINGS SHALL BE 50 DUROMETER HARDNESS.

DEAD LOAD DEFLECTION AT	ND CAMBER					
	$3'-0'' \times 1'-9''$					
45' CORED SLAB UNIT	0.6″∅ L.R. STRAND					
CAMBER (SLAB ALONE IN PLACE)	7∕8″ ੈ					
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD**	1/8″ ♦					
FINAL CAMBER	3/4″ ♠					
** INCLUDES FUTURE WEARING SURFACE						

END VIEW



	BILL OF MATERIAL FOR ONE 45' CORED SLAB UNIT										
EXTERIOR UNIT INTERIOR UNIT											
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT				
B5	4	#4	STR	23'-3"	62	23'-3"	62				
S1	8	#5	3	4'-6"	38	4'-6"	38				
S2	122	#4	3	5′-4″	435	5'-4"	435				
* S3	54	#5	1	5'-7"	314						
S5	4	#4	3	5′-5″	14	5′-5″	14				
S6	4	#4	3	5′-6″	15	5′-6″	15				
S7	4	#4	3	5'-7"	15	5′-7″	15				
S8	4	#4	3	5′-9″	15	5′-9″	15				
REINF	ORCING S	STEEL	LBS	<u> </u>	594		594				
₩ EPOX REIN	XY COATE NFORCINC	ED Steel	LB:	S.	314						
6500	P.S.I.CO	NCRETE	CU. YDS)	6.6		6.6				

GUTTERLINE ASPI	HALT THICKNESS & RAI	L HEIGHT
	ASPHALT OVERLAY THICKNESS	RAIL HEIGHT
	@ MID-SPAN	@ MID-SPAN
45' UNITS	2"	3′-8″

0.6″∅ L.R. STRANDS

END OF RAIL DETAILS

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE $2\frac{1}{2}$ " \varnothing dowel holes at fixed ends of slab sections shall be FILLED WITH NON-SHRINK GROUT.

THE $2\frac{1}{2}$ \emptyset dowel holes at expansion ends of slab sections shall BE FILLED WITH EXPANSION JOINT MATERIAL.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

ALL REINFORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

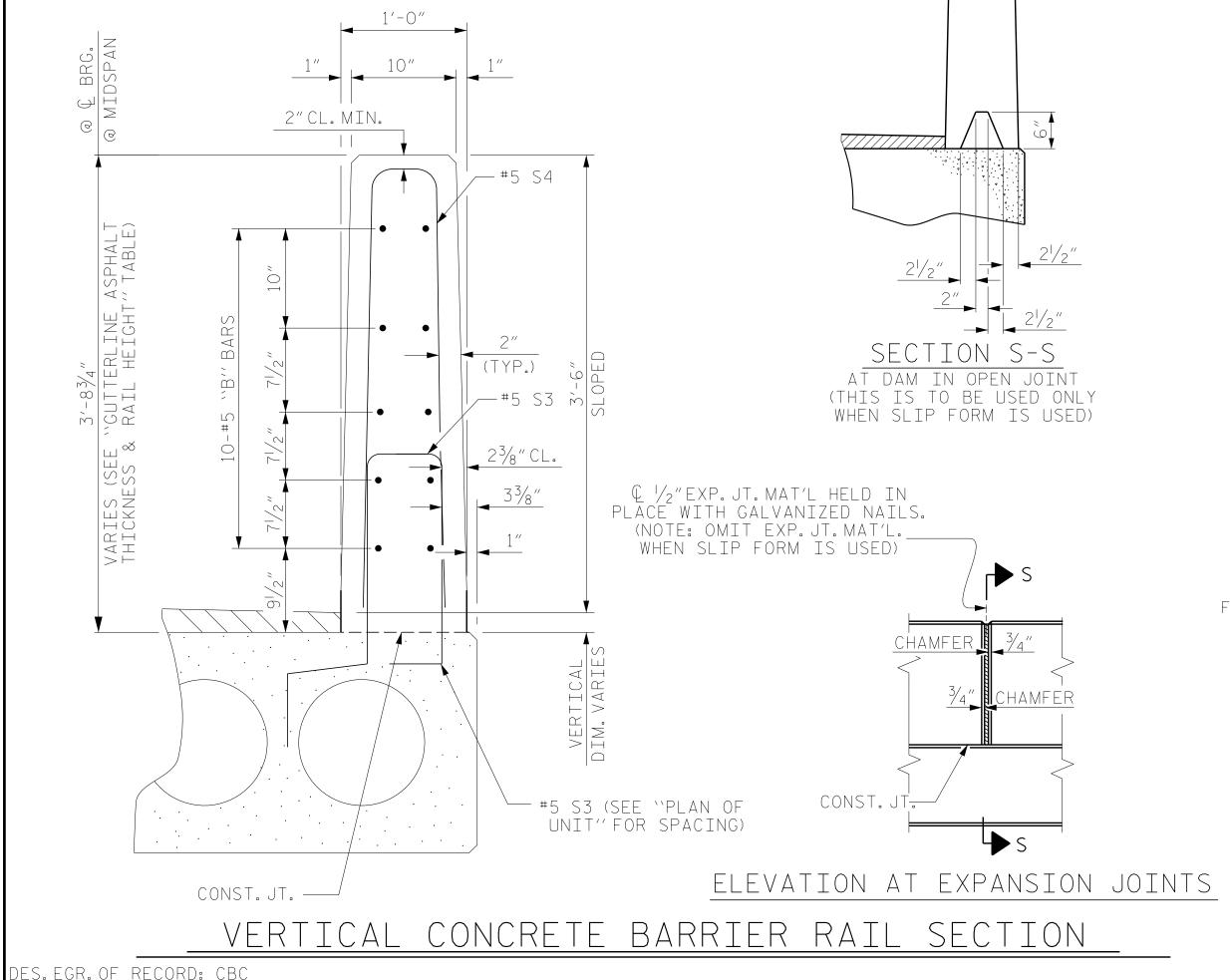
FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-O"CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.



DATE: 3/2015

DATE: 3/2015

RWW

HLW

DRAWN BY :

CHECKED BY :

CORED SLABS REQUIRED

EXTERIOR C.S.| 2 | 45'-0"|

INTERIOR C.S. 9 45'-0"

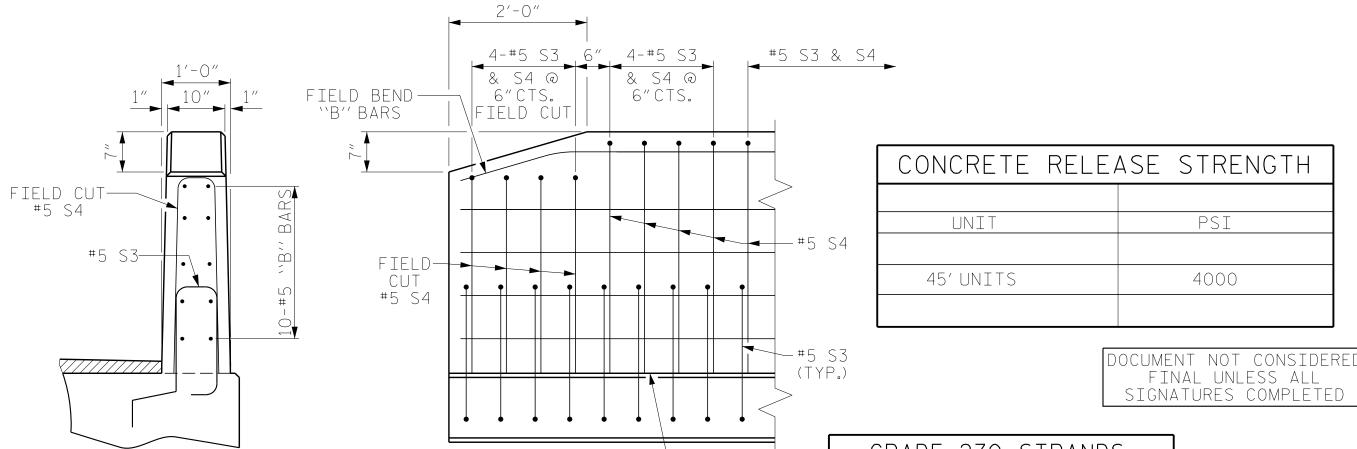
45' UNIT

NUMBER LENGTH TOTAL LENGT

90′-0″

405'-0"

495'-0"



CONST. JT.

SIDE VIEW

GRADE 270 STRANDS 0.6″Ø L.R.

0.217 SQUARE INCHES JLTIMATE STRENG (LBS. PER STRAND 58,600 APPLIED PRESTRES 43,950 LBS. PER STRAND



PSI

4000

PROJECT NO. 17BP.14.R.143 TRANSYLVANIA COUNTY STATION: 15+90.36 -L-

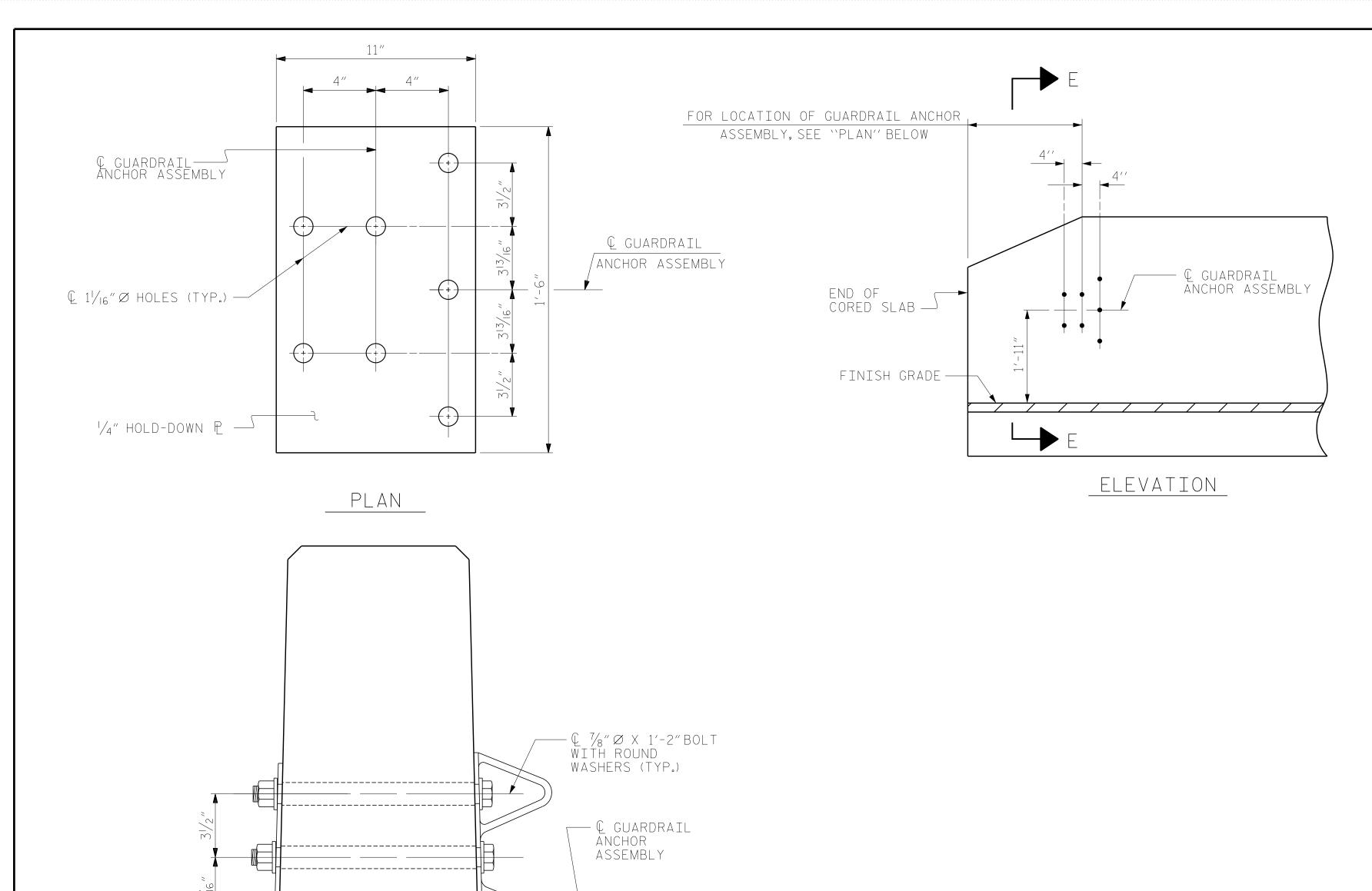
SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

STANDARD

3'-0'' X 1'-9'' CORED SLAB UNIT

		REVI	SION	IS		SHEET NO.
10.	BY:	DATE:	NO.	BY:	DATE:	S-7
1			3			TOTAL SHEETS
2			4			17



NOTES (FOR METAL RAILS)

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $1/4^{\prime\prime}$ HOLD DOWN PLATE AND 7 - $1/8^{\prime\prime}$ Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36.AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE $\frac{1}{8}$ $\frac{1}$

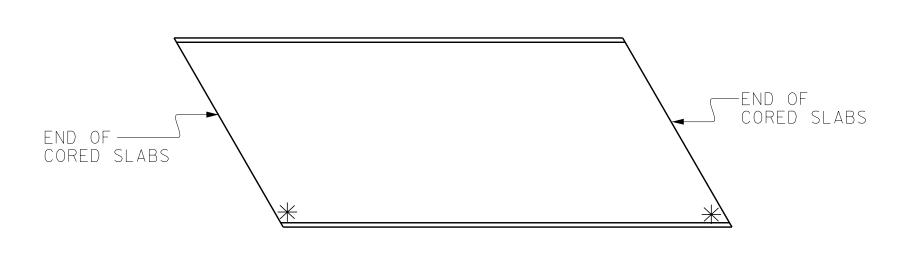
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF THE PARAPET. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLIES WITH BOLTS, NUTS AND WASHERS COMPLETE IN PLACE, SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

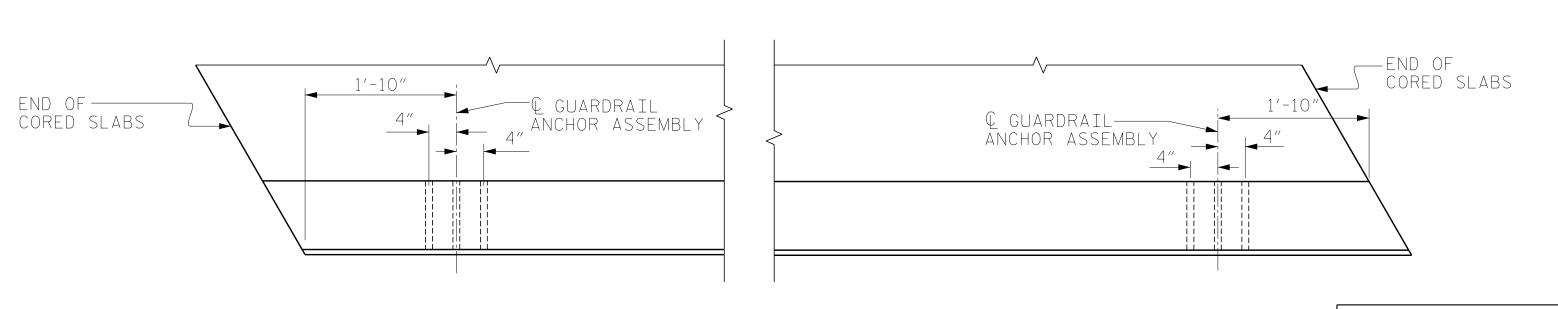
THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE END POST TO CLEAR ASSEMBLY BOLTS.

THE 1 $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



SKETCH SHOWING POINTS OF ATTACHMENT

*LOCATION OF GUARDRAIL ATTACHMENT



END BENT #1

PLAN

END BENT #2

LOCATION OF GUARDRAIL ANCHOR AT END POST

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

SEAL P. 20777

PROJECT NO. 17BP.14.R.143

TRANSYLVANIA COUNTY

STATION: 15+90.36 -L-

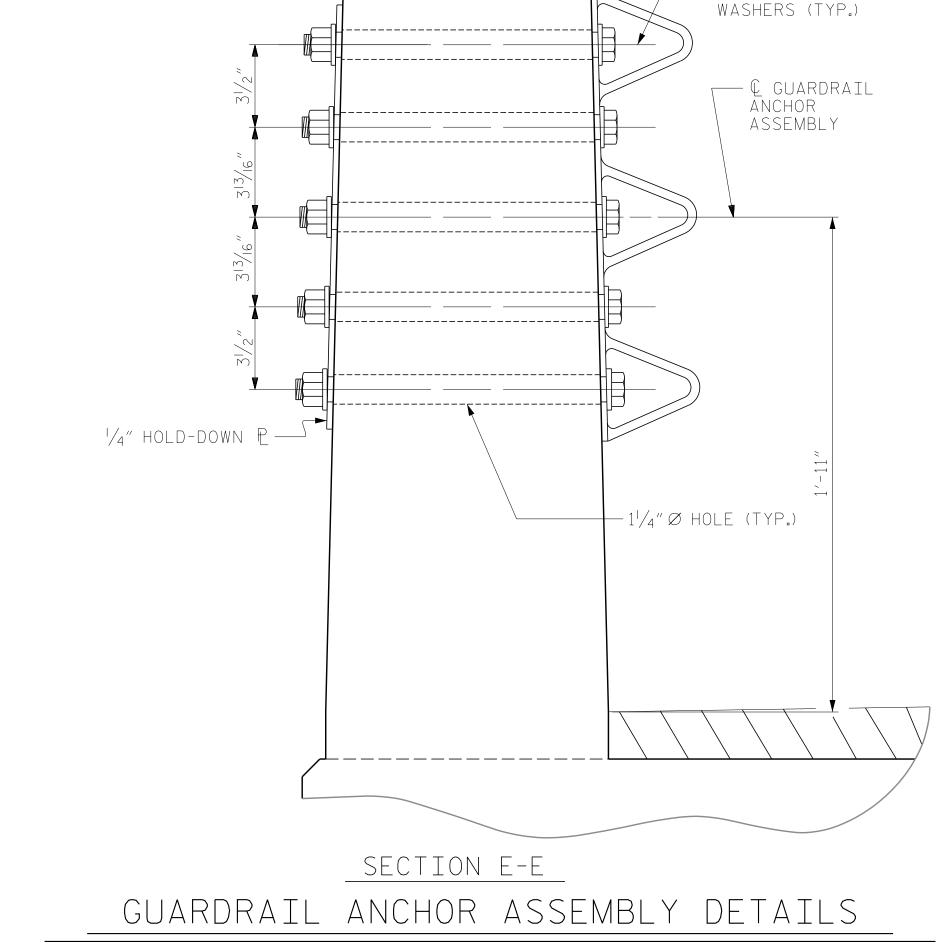
STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

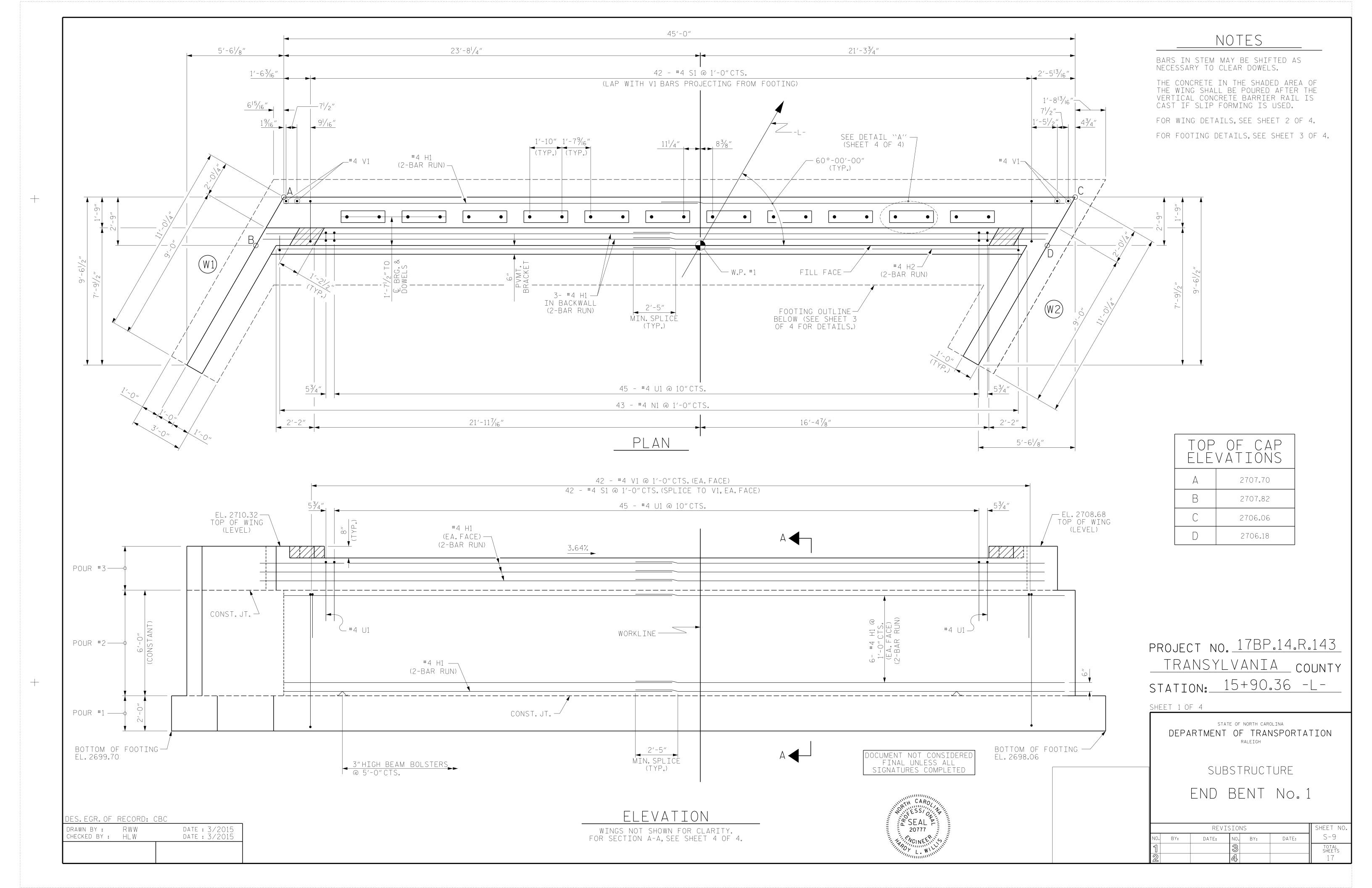
GUARDRAIL ANCHORAGE VERTICAL CONCRETE BARRIER RAIL

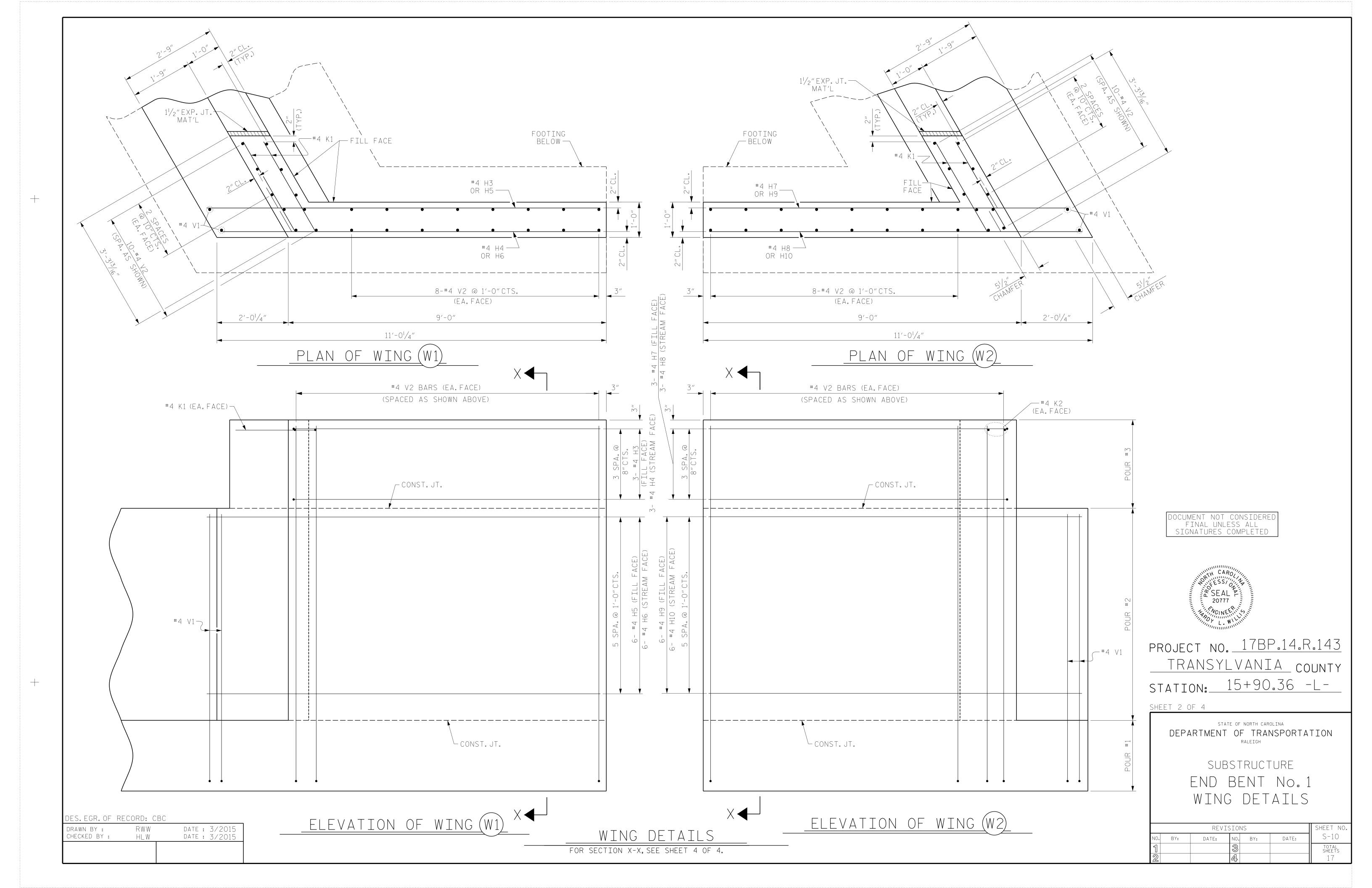
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BY:	DATE:	NO.	BY:	DATE:	S-8					
		3			TOTAL SHEETS					
		4			17					

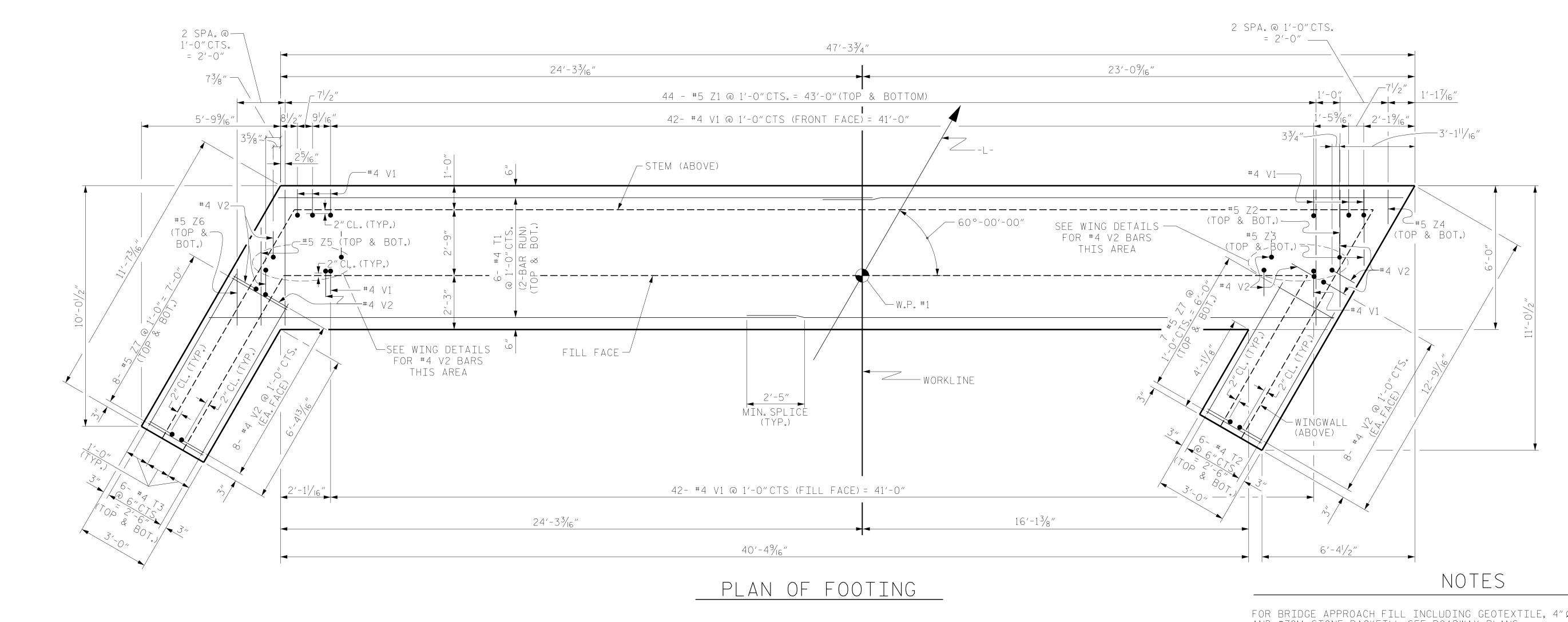
(SHT. 16) STD. NO. GRA3

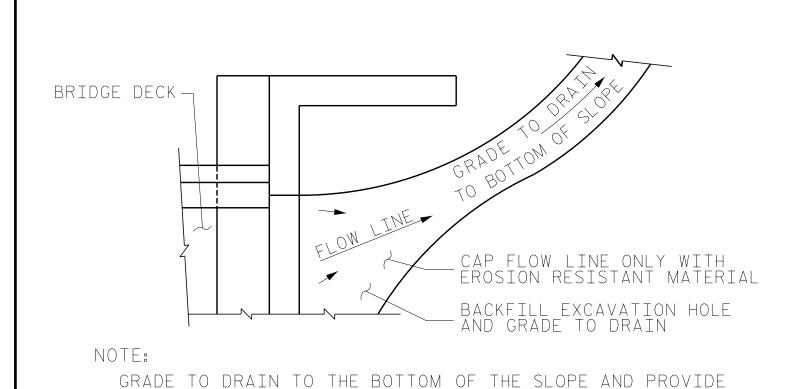


DRAWN BY: RWW DATE: 3/2015
CHECKED BY: MAA 5/10
CHECKED BY: GM 5/10
REV. 10/1/II MAA/GM
REV. 12/5/II MAA/GM
REV. 6/13 MAA/GM









TEMPORARY DRAINAGE DETAIL

DATE: 3/2015

DATE: 3/2015

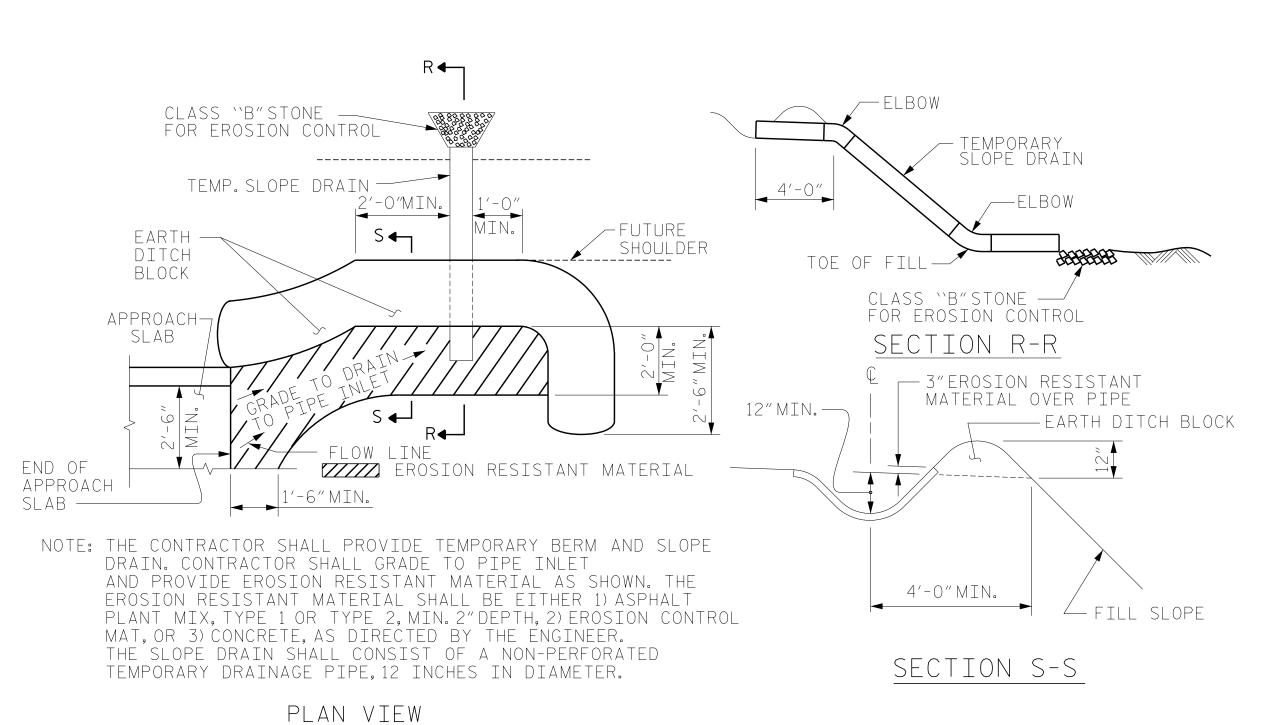
DES.EGR.OF RECORD: CBC

DRAWN BY: RWW

CHECKED BY: HLW

EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING

OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE.



FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, 4" Ø DRAINAGE PIPE, AND #78M STONE BACKFILL, SEE ROADWAY PLANS.

GEOTEXTILE SHALL BE TYPE 1IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.

#78M STONE BACKFILL (CLASS V SELECT MATERIAL) SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.

#78M STONE BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL .

FOR THE 4" Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD DRAWINGS. THE CONTRACTOR SHALL ENSURE THAT DRAIN PIPE OUTFALL IS ABOVE STREAM ELEVATION.

AREA BETWEEN THE WINGWALL AND EDGE OF PAVEMENT SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

FOR DRAINAGE DETAIL BEHIND END BENT, SEE SHEET 4 OF 4.

OCUMENT NOT CONSIDERED

FINAL UNLESS ALL Signatures completed

SEAL

20777

PROJECT NO. <u>178P.14.R.143</u> TRANSYLVANIA COUNTY STATION: 15+90.36 -L-

SHEET 3 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

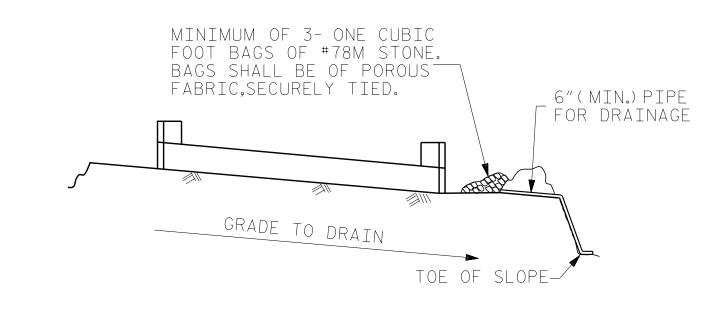
SUBSTRUCTURE

END BENT No. 1

		SHEET NO.				
٥.	BY:	DATE:	NO.	BY:	DATE:	S-11
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			4			17

TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)

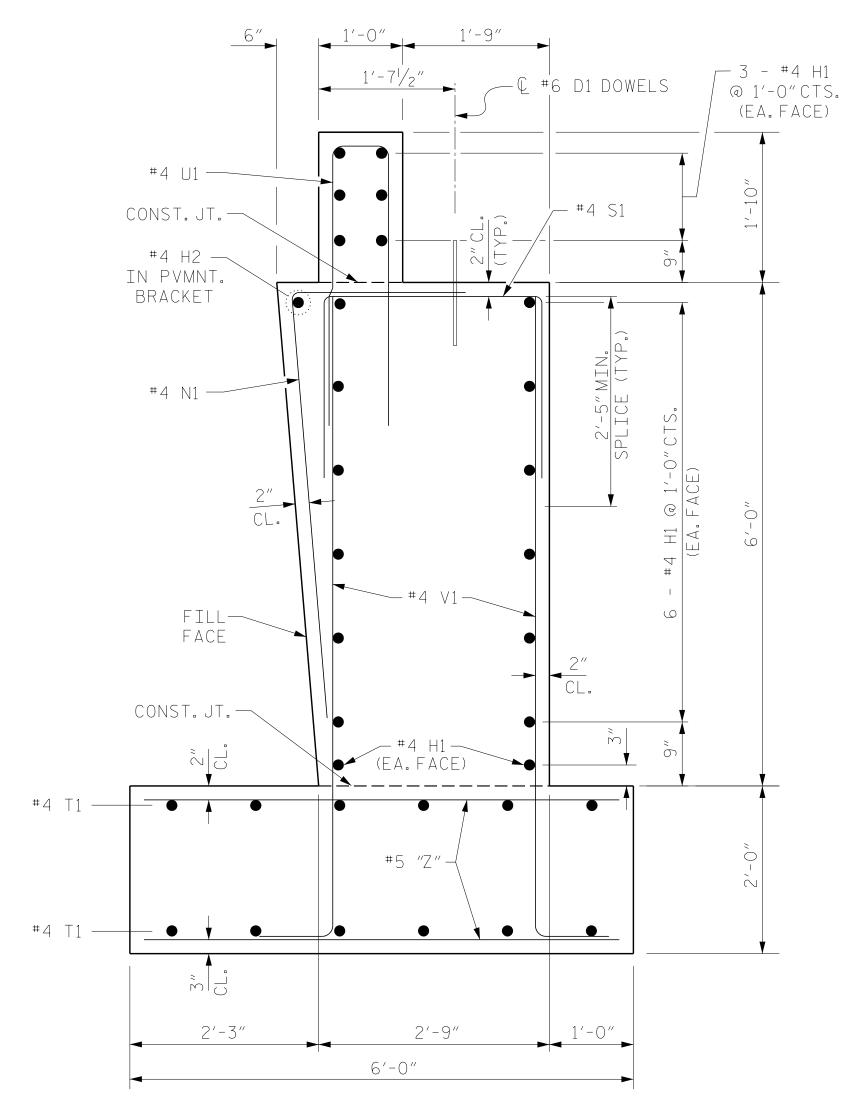


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

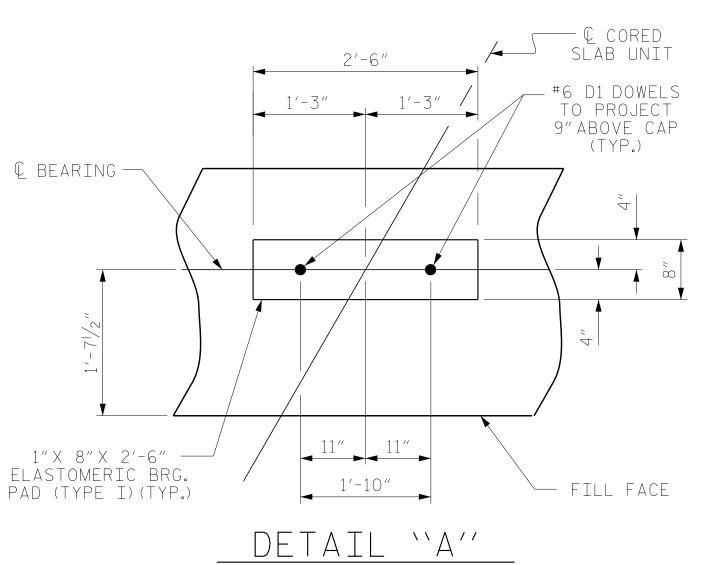
TEMPORARY DRAINAGE AT END BENT



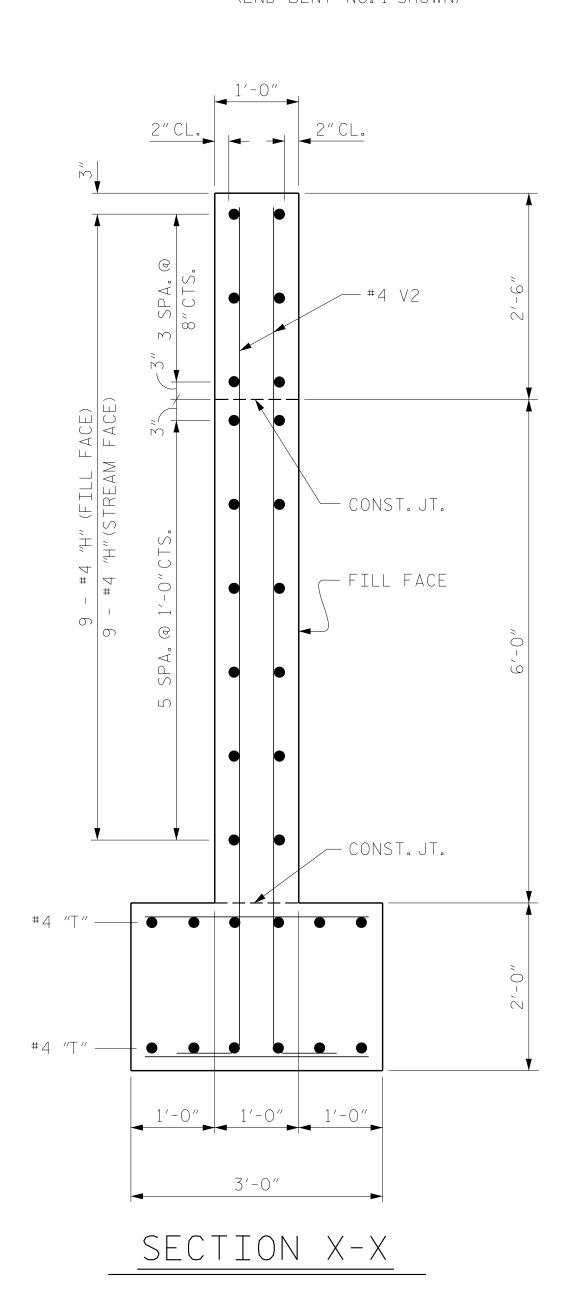
<u>section a-a</u>

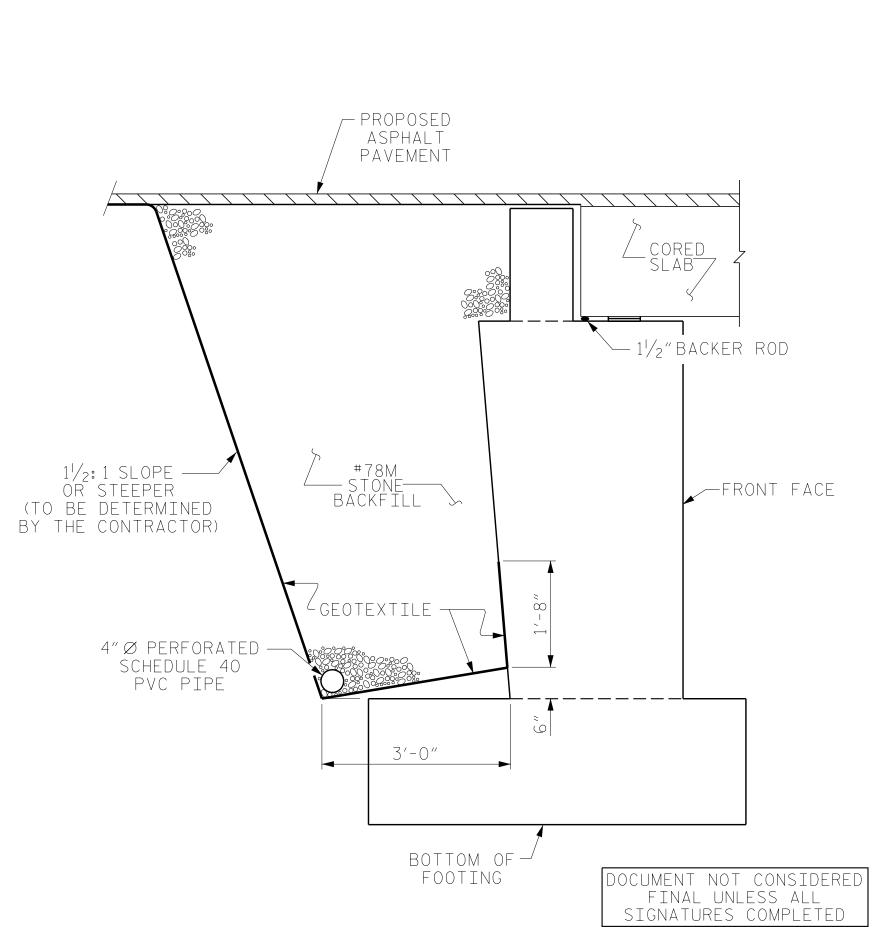
DES.EGR.OF RECORD: CBC

DRAWN BY: RWW DATE: 3/2015
CHECKED BY: HLW DATE: 3/2015



(END BENT No.1 SHOWN)





BAR TYPES

(5)

8'-2"

8'-5"

ALL BAR DIMENSIONS ARE OUT TO OUT.

9'-0"

8'-9"

5′-10″

(3)

3"Ø-(TYP.)

DRAINAGE DETAIL

END BENT NO.1 SHOWN, END BENT NO.2 SIMILAR.



]] 0	1 1 2
H4	3	#4	4	9'-5"	19
H5	6	#4	STR	11'-1"	44
Н6	6	#4	STR	10'-9"	43
H7	3	#4	5	8'-10"	18
H8	3	#4	5	9'-1"	18
H9	6	#4	STR	10'-2"	41
H10	6	#4	STR	10'-6"	42
K1	4	#4	STR	2'-11"	8
N1	43	#4	1	7'-11"	227
S1	42	#4	2	7′-5″	208
T1	24	#4	STR	24'-8"	395
Т2	12	#4	STR	6'-4"	51
Т3	12	#4	STR	7'-4"	59
U1	47	#4	2	7′-5″	233
V1	88	#4	3	8'-1"	475
٧2	52	#4	3	8'-9"	304
Z1	88	#5	STR	5′-8″	520
Z2	2	#5	STR	4'-10"	10
Z3	2	#5	STR	3'-2"	7
Z 4	2	#5	STR	1'-5"	3
Z5	2	#5	STR	4'-1"	9
Z6	2	#5	STR	2'-4"	5
Z7	30	#5	STR	2'-8"	83
REIN	FORCI	ng ste	EL	-	3551 LBS.

BILL OF MATERIAL

BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT

D1 22 #6 STR 1'-6"

H1 40 #4 STR 23'-7"

H2 2 #4 STR 22'-5"

H3 3 #4 4 9'-8"

FOR END BENT #1

50

630

30

19

23.4 C.Y

61.5 C.Y.

PROJECT NO. 17BP.14.R.143

TRANSYLVANIA COUNTY

STATION: 15+90.36 -L-

CLASS A CONCRETE BREAKDOWN

TOTAL CLASS A CONCRETE

POUR #2 STEM AND LOWER PART 33.4 C.Y

POUR #3 BACKWALL AND UPPER 4.7 C.Y. PART OF WINGS

POUR #1 FOOTING

SHEET 4 OF 4

STATE OF NORTH CAROLINA

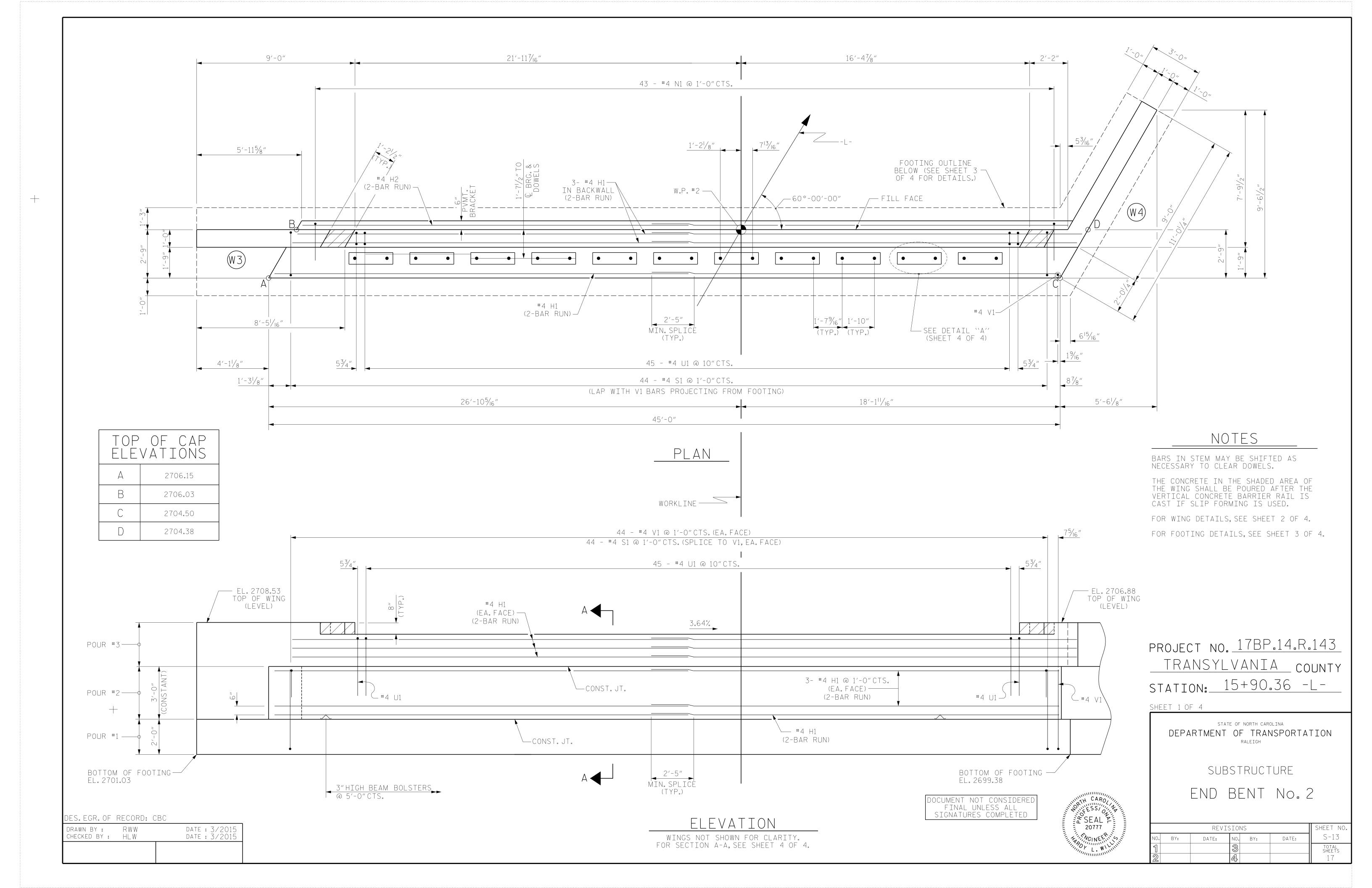
DEPARTMENT OF TRANSPORTATION

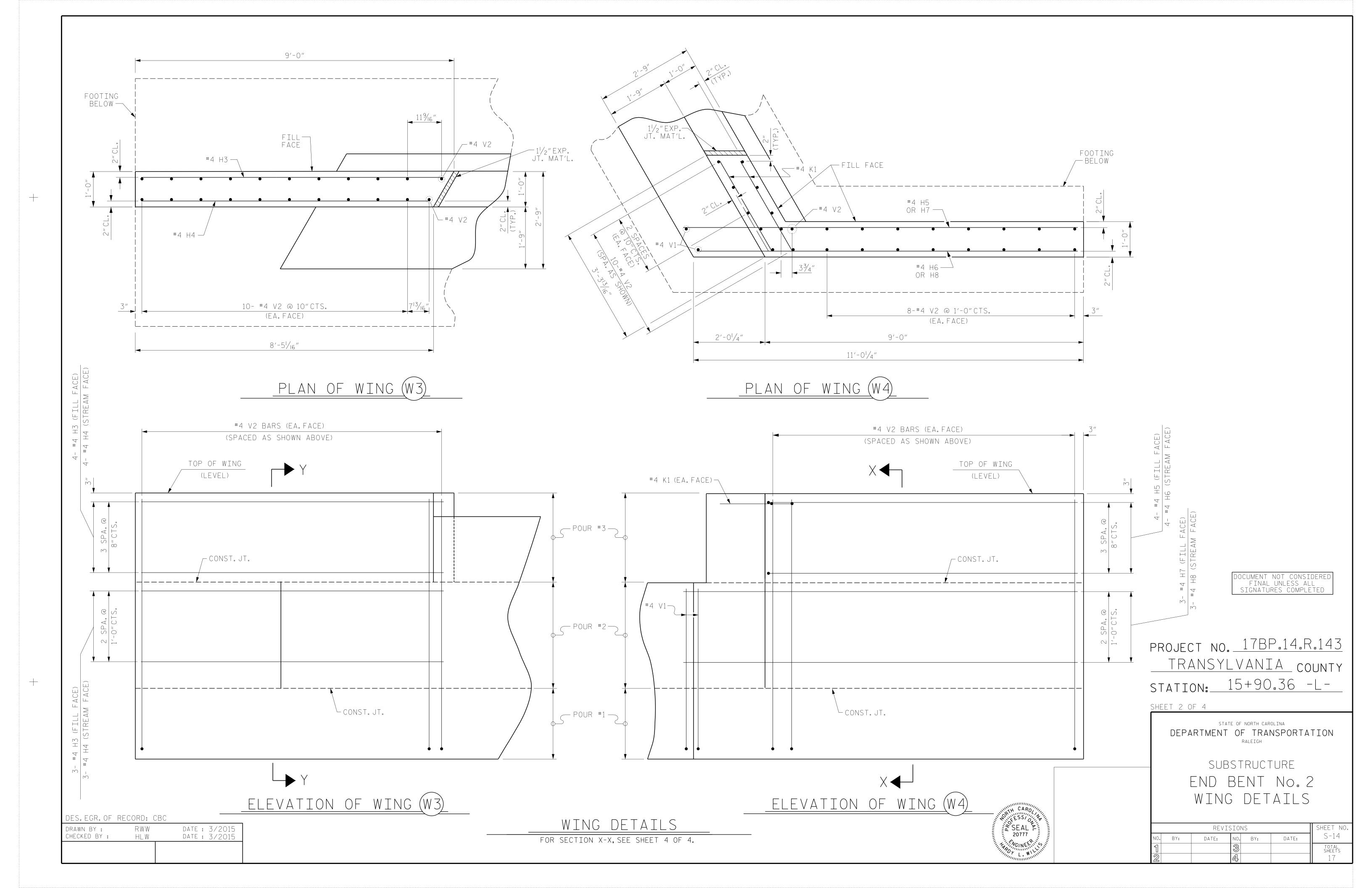
RALEIGH

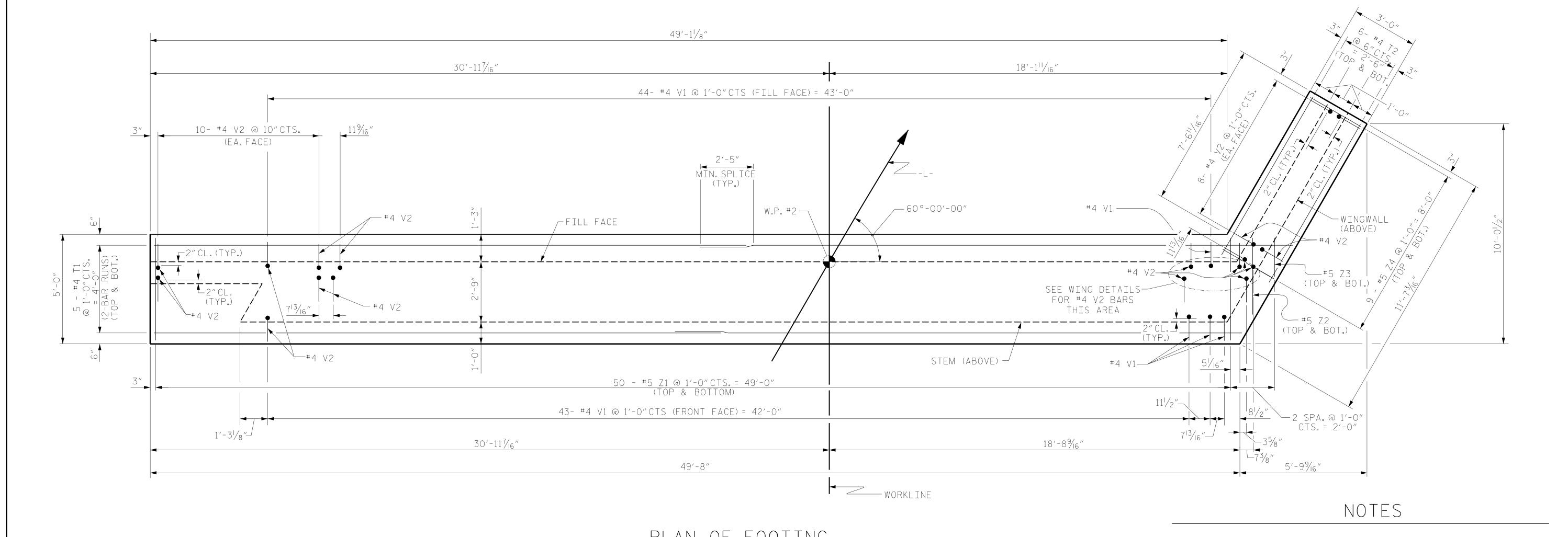
SUBSTRUCTURE

END BENT No.1 DETAILS

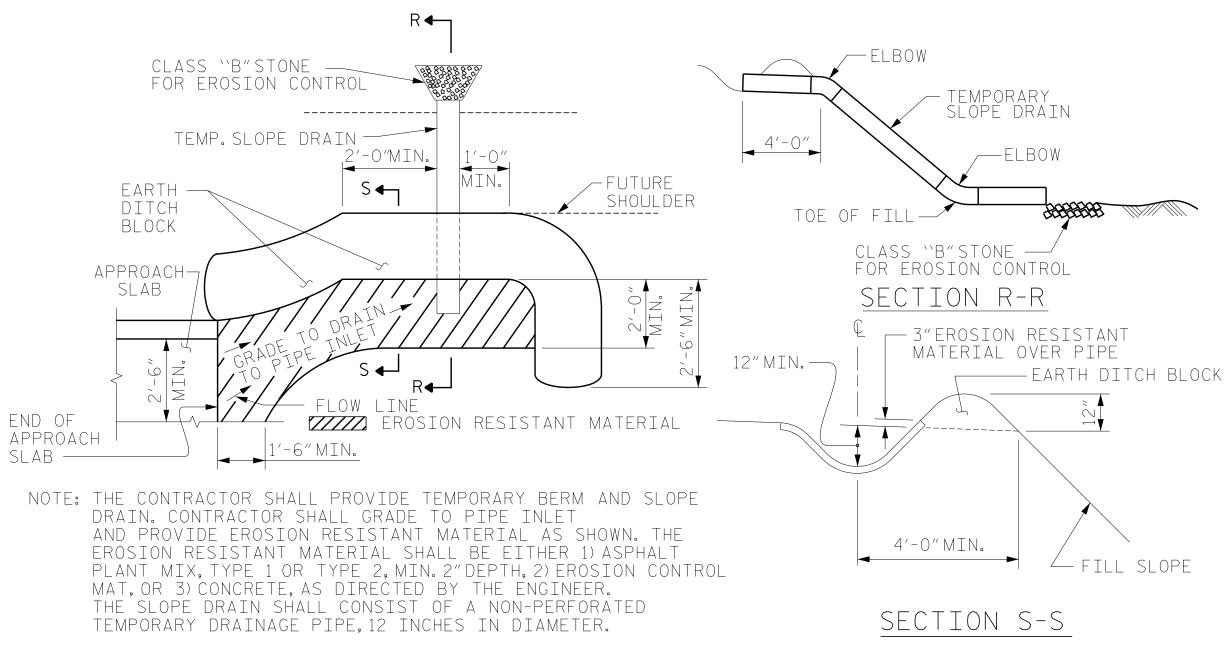
	SHEET NO.				
BY:	DATE:	NO.	BY:	DATE:	S-12
		3			TOTAL SHEETS
		4			17







PLAN OF FOOTING



FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, 4" Ø DRAINAGE PIPE, AND #78M STONE BACKFILL, SEE ROADWAY PLANS.

GEOTEXTILE SHALL BE TYPE 1IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.

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AREA BETWEEN THE WINGWALL AND EDGE OF PAVEMENT SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

FOR DRAINAGE DETAIL BEHIND END BENT, SEE END BENT NO. 1 (SHEET 4 OF 4).

PROJECT NO. <u>178P.14.R.143</u> TRANSYLVANIA COUNTY STATION: 15+90.36 -L-

SHEET 3 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

SUBSTRUCTURE

END BENT No. 2

SHEET NO REVISIONS S-15 DATE: BY: DATE: 10. BY: TOTAL SHEETS

TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)

PLAN VIEW

DES. EGR. OF RECORD: CBC DRAWN BY: RWW DATE: 3/2015 CHECKED BY: HLW DATE: 3/2015

1 FLOW LINE

GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING

OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE.

TEMPORARY DRAINAGE DETAIL

CAP FLOW LINE ONLY WITH EROSION RESISTANT MATERIAL

BACKFILL EXCAVATION HOLE AND GRADE TO DRAIN

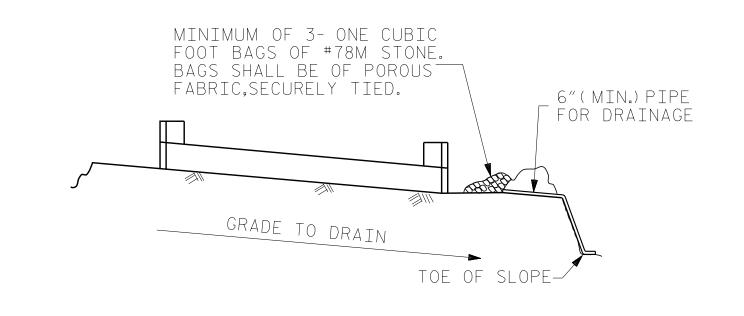
BRIDGE DECK -

NOTE:



OCUMENT NOT CONSIDEREI

FINAL UNLESS ALL SIGNATURES COMPLETED

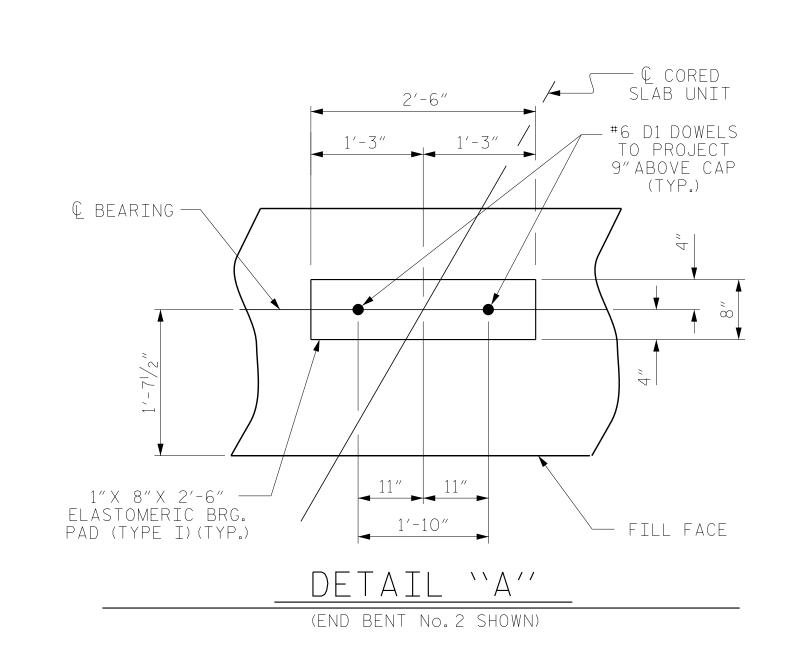


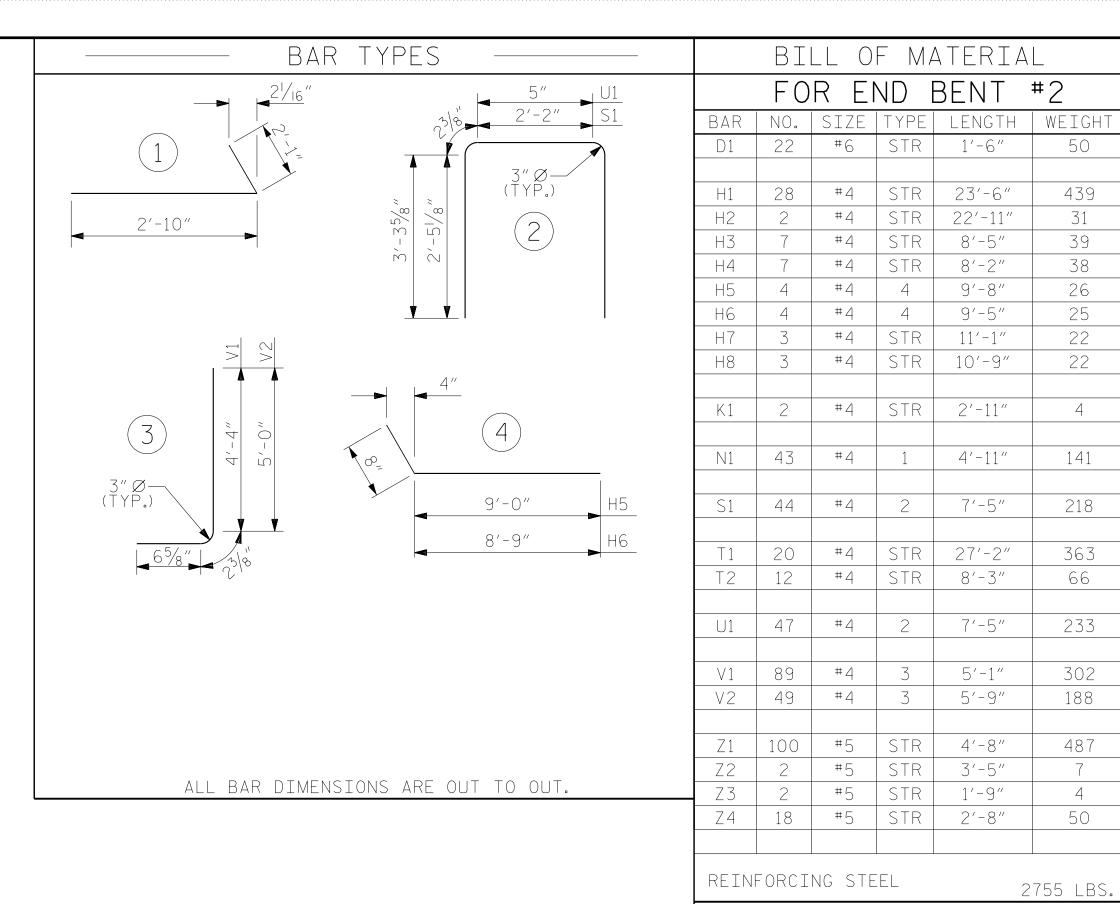
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TEMPORARY DRAINAGE AT END BENT





3 - +4 HI
@ I'-0° CTS.
(EA, FACE)

#4 S1

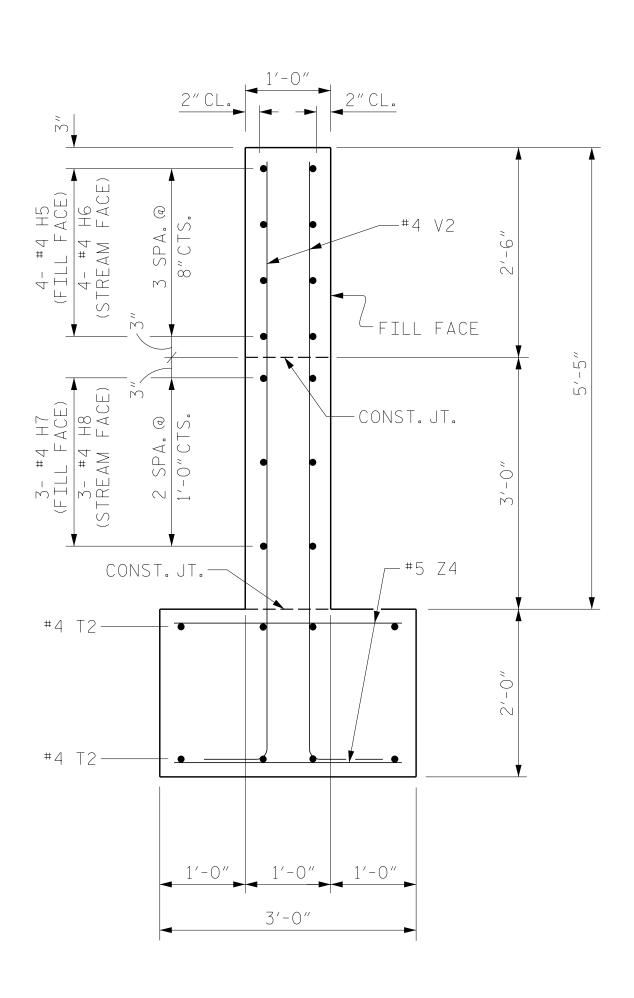
#4

SECTION A-A

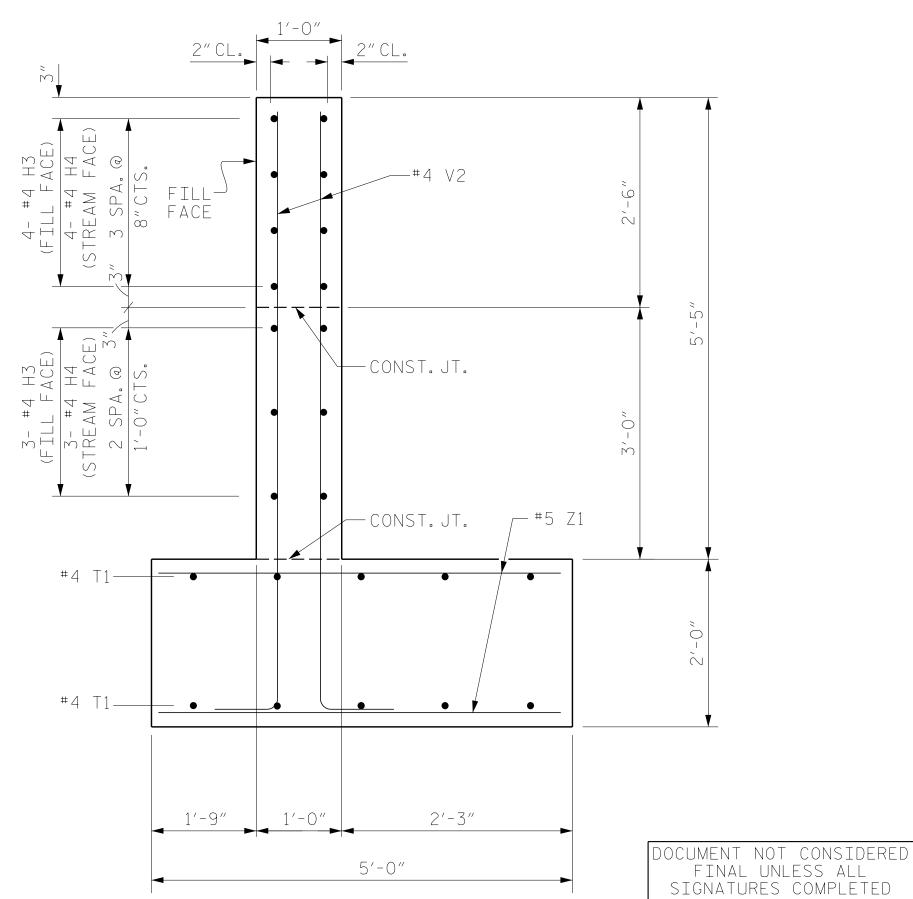
DATE: 3/2015 DATE: 3/2015

DES.EGR.OF RECORD: CBC

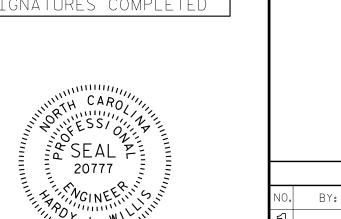
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SECTION X-X



<u>SECTION Y-Y</u>



PROJECT NO. 17BP.14.R.143

TRANSYLVANIA COUNTY

STATION: 15+90.36 -L-

CLASS A CONCRETE BREAKDOWN

OF WINGS

TOTAL CLASS A CONCRETE

POUR #2 STEM AND LOWER PART 16.5 C.Y.

POUR #3 BACKWALL AND UPPER 4.5 C.Y.

PART OF WINGS

20.4 C.Y

41.4 C.Y.

POUR #1 FOOTING

SHEET 4 OF 4

STATE OF NORTH CAROLINA

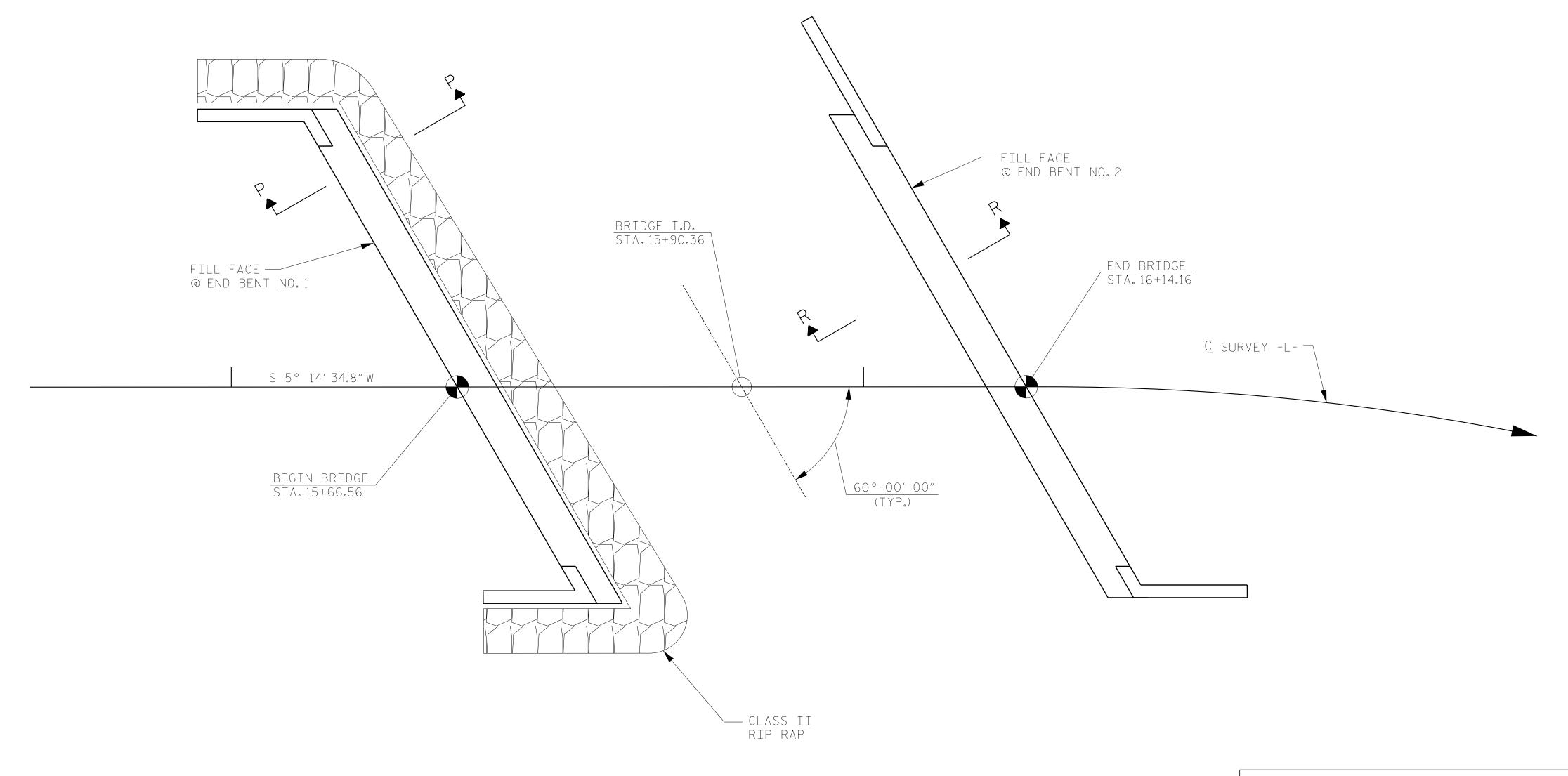
DEPARTMENT OF TRANSPORTATION

RALEIGH

SUBSTRUCTURE

END BENT No. 2 DETAILS

REVISIONS					SHEET NO.	
NO.	BY:	DATE:	NO.	BY:	DATE:	S-16
1			3			TOTAL SHEETS
2			4			17



ESTIMATED QUANTITIES				
BRIDGE @ STA.15+90.36	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE		
	TONS	SQUARE YARDS		
END BENT 1	32	36		
END BENT 2	-	-		

PROJECT NO. <u>178P.14.R.143</u> TRANSYLVANIA_ COUNTY STATION: 15+90.36 -L-

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RALEIGH

STANDARD

---RIP RAP DETAILS---

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED





SECTION R-R

GROUND LINE

SECTION P-P

GROUND LINE—

TOP OF CLASS II RIP RAP

EL. 2703.30

GEOTEXTILE

DES.EGR.OF RECORD: CBC

DATE: 3/2015 DATE: 3/2015 DRAWN BY : Checked by :

SHEET NO REVISIONS S-17 DATE: DATE: BY: NO. BY: TOTAL SHEETS

STANDARD NOTES

DESIGN DATA:

SPECIFICATIONS ---- A.A.S.H.T.O. (CURRENT) LIVE LOAD ---- SEE PLANS IMPACT ALLOWANCE ----- SEE A.A.S.H.T.O. STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 - 20,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50W - 27,000 LBS.PER SQ.IN. - AASHTO M270 GRADE 50 - 27.000 LBS.PER SQ.IN. REINFORCING STEEL IN TENSION GRADE 60 - - 24.000 LBS. PER SQ. IN. CONCRETE IN COMPRESSION ----- 1,200 LBS. PER SQ. IN. CONCRETE IN SHEAR STRUCTURAL TIMBER - TREATED OR UNTREATED - EXTREME FIBER STRESS - - - - - 1.800 LBS. PER SQ. IN. COMPRESSION PERPENDICULAR TO GRAIN 375 LBS.PER SQ.IN. OF TIMBER - - - -EQUIVALENT FLUID PRESSURE OF EARTH ----30 LBS.PER CU.FT.

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2006 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

(MINIMUM)

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS.

SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8% SHEAR STUDS FOR THE 3/4% STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3-7/8% STUDS FOR 4-3/4% STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8% STUDS ALONG THE BEAM AS SHOWN FOR 3/4% STUDS BASED ON THE RATIO OF 3-7/8% STUDS FOR 4-3/4% STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0%.

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH

JANUARY, 1990